Reinforcing Rabric in Buildings

Triangle Mesh Fabric Electric Weld Fabric

American Steel & Wire Company



218-11.

Wire Fabric Concrete Reinforcement

ILLUSTRATIONS SUGGESTIONS AND TABLES COVERING ITS USE IN BUILDING CONSTRUCTION

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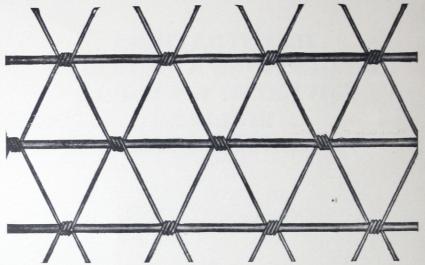
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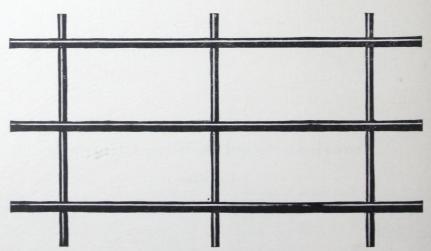
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Triangle Mesh Reinforcement.



American Electric Weld Fabric.

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Triangle Mesh Reinforcement and American Electric Weld Fabric can be economically used in some form in every building constructed.

Use it to reinforce the concrete floors and roofs, the concrete walls (interior and exterior),

the basement floors and other floors that rest upon the ground,

the concrete wearing course over concrete floors,

the concrete fireproofing of all steel frame work,

the concrete side walks, drive ways, streets and highways,

the concrete sewers,
the concrete coal bins,
the concrete silo,
the concrete water tanks,
the concrete oil tanks,
the concrete ships and barges,
the revetment in river work,
the concrete lining in reservoirs,
the concrete chimney,
the flat slab beamless floors,
the retaining walls.

Introductory

THIS catalog contains a description of wire fabric concrete reinforcement and illustrates and explains its use in various types of building construction.

For many classes of work wire fabric is more efficient and more economical than loose bars.

Wire is a product resulting from drawing a hot rolled rod through a die which mechanically reduces the size and increases the length. The yield point and the ultimate strength is increased 50 to 100 per cent depending upon the amount of drawing or reduction. The finished wire is then partially annealed to reduce the stiffness thereby making a fabric of high yield point and tensile strength that is still sufficiently flexible to insure ease in handling.

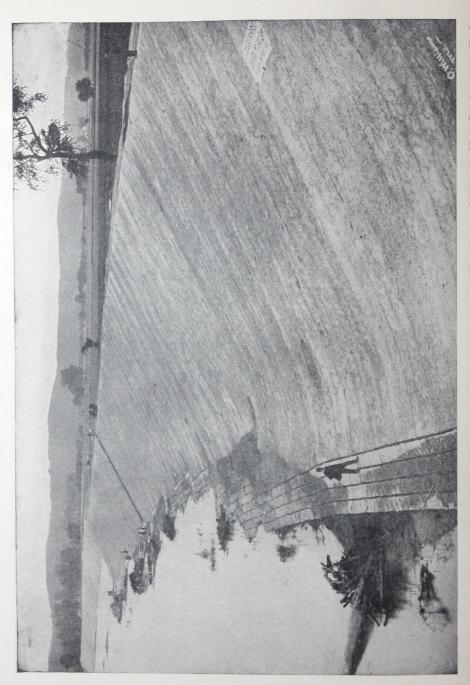
The several wires making up the fabric are not injured in any way nor is the yield point or tensile strength reduced during the process of manufacture.

The yield point of the wire is at least 50 per cent greater than the yield point of reinforcing bars; therefore, based on strength of the steel alone, wire fabric is more efficient by 50 per cent than bars of equivalent area.

The bonding area of wire fabric is several times greater than that for a bar of an equal cross sectional area. This can be shown by the simple example of comparing the bond area or surface of four ½-inch square bars with that for one 1-inch square bar which has a sectional area equal to the four ½-inch bars. The four ½-inch bars have a total bonding surface of twice that for the 1-inch bar. Comparing sixteen ¼-inch square bars with one 1-inch bar shows the total bonding surface to be four times that for the 1-inch bar.

This explains in part why better results are obtained with a close meshed fabric than with bars.

When comparing costs of wire fabric and bars for any particular job, the comparison is not complete unless the costs to install are included. For the average floor job wire fabric can be installed in the forms at a cost of about 10 to 20 per cent of that for bars. See P. 9 for comparative designs of a typical floor panel.



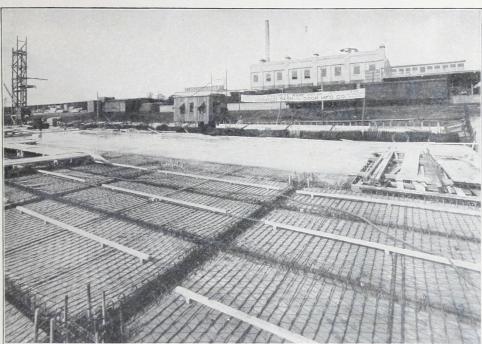


Fig. 1. Triangle Mesh Reinforcement used in the concrete floor slabs of the Norwood Sash & Door Co., Norwood, Ohio.

Note its use with concrete supporting beams.

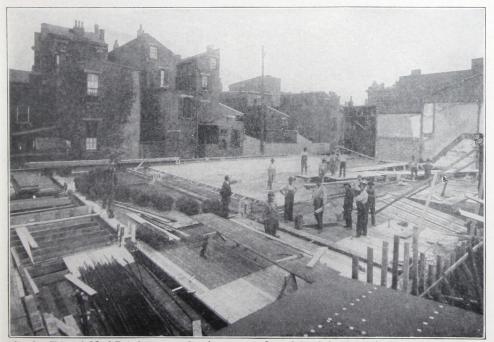


Fig. 2. Triangle Mesh Reinforcement for the concrete floor slabs of the Shillitoh Stables, Cincinnati, O. Note its use with concrete supporting beams and note particularly the ease with which the mesh reinforcement is placed in position.

Wire Fabric Reinforcement for Short Span Slabs Supported by Concrete Beams

The short span type of floor construction with its reinforced concrete slabs supported by reinforced concrete beams, girders and columns has demonstrated its superiority over the long span type for many kinds of buildings.

As a general rule three spans per panel (Fig. 3) wili be more economical for the average building than two spans per panel (Fig. 4), but an estimate of the cost per panel of the two types should be figured before deciding definitely which shall be used.

Wire Fabric Reinforcement is the most economical form of slab reinforcement as can easily be demonstrated by making a comparative estimate. Even with high priced labor the placing cost will not exceed \$2.00 per ton for the mesh as compared with \$15.00 to \$20.00 per ton for equivalent bars.

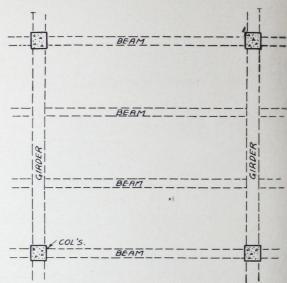


Fig. 3. Three spans per panel.

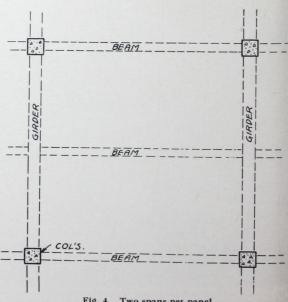


Fig. 4. Two spans per panel.

Design Suggestions, Cost, Etc.

From an economical standpoint it is very desirable to have the same amount of reinforcement and the same thickness of slab in the end as in the intermediate slabs; the inspection requirements are less and in fact the entire installation costs are reduced. This is true whether bars or mesh constitute the slab reinforcement. The construction of the forms can be seldom made exactly uniform for the end and intermediate panels regardless of the relative lengths of spans "A" and "B" therefore the cost of forms should not be considered.

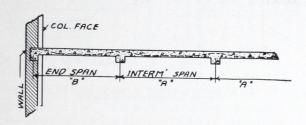


Fig. 5. Uniform reinforcement and thickness of slab in intermediate and end slabs. Span "B" must not be greater than 9/10X"A."

In order for span "B" to have resisting moments equal to span "A" assuming the same reinforcement and same thickness of slab in both cases the length of span "B" must not be more than $\frac{9}{10}$ times "A."

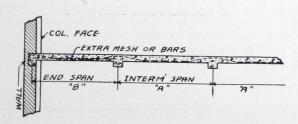


Fig. 6. Scheme for increasing the reinforcement in the end spans.

Under certain conditions it may be found advisable to so space the columns as to require a greater or less amount of reinforcement or a greater or less thickness of slab for the end spans than for the intermediate spans. If the reinforcement requirements are greater add sufficient bars or another layer of mesh to make up the difference. We recommend the spacing of these bars to be not more than 24 inches.

Supporting Wire Fabric Reinforcement Over Concrete Beams

The correct location of the reinforcing mesh in the top portion of slabs over concrete beams can be easily and cheaply obtained by means of precast concrete blocks as shown in Fig 7.

These blocks should have a height about $1\frac{1}{4}$ or $1\frac{1}{2}$ inches less than the full thickness of the slab. The length may be any convenient amount such as one or

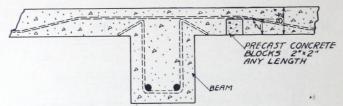


Fig. 7. Showing inexpensive method of supporting Wire Fabric Reinforcement over concrete beams.

two feet or such lengths as readily form by breaking long strips of the blocks. The mesh should be supported for at least one third of its width. These blocks will stay in approximately correct position without any means of attachment to the reinforcing mesh or to the forms.

For short spans (about 8 feet or less) these blocks will be required on one side only of the beams. For spans greater than 8 feet use blocks on both sides.

Detailing Widths of Mesh

For estimating purposes where accuracy is unnecessary provided the estimate is on the safe side it is permissible to assume that the mesh required will equal in square feet the actual area of the floor slab; as a matter of fact the amount of mesh needed will be less than that, and at the same time a more efficient and more economical layout is possible. In the first place a two inch lap along the

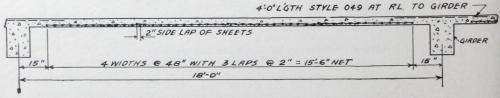
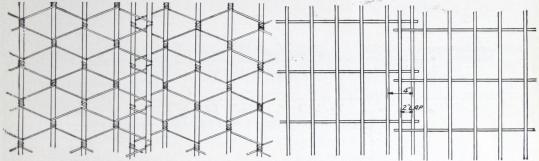


Fig. 8. Detailing Widths of Mesh and showing extra strip of mesh over the girders.

sides of the sheets is sufficient to develop the full strength of the reinforcement; more than that is a waste of material. This two inch lap means a lap of the outside (or selvage) longitudinal wires as shown in Fig. 9. The cross wires that extend beyond the longitudinals are not counted in as part of the lap.

A suggested detail for the amount and location of Triangle Mesh Reinforcement is shown in Fig. 8. Providing a short strip of light weight fabric (style 049 or 053P) is placed near the top of the slab over the girders with the main longitudinal wires at right angles to the direction of the girders it is not only unnecessary but a waste of material to require that the main reinforcement of



Triangle Mesh Reinforcement.

American Electrically Welded Fabric.

Fig. 9. Shows Wire Fabric as it actually appears with a side lap of 2 inches.

the slabs entirely cover the space between and over the girders. The distance between the center line of the girder and the edge of the reinforcing fabric can be any amount up to about 20 inches without reducing the strength of the structure.

Comparative Designs

The following designs show conclusively that Triangle Mesh Reinforcement is more economical than bars for a typical short span slab. Assuming average

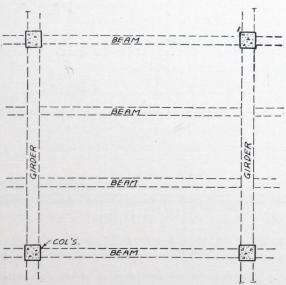


Fig. 10. Showing location of beams and girders in typical panel.

conditions the cost of placing the mesh in the forms ready for concreting will not exceed \$2.00 per ton while the cost of bending and placing bars will be at least \$15.00 per ton and very often it exceeds this by \$5.00 or \$10.00 per ton.

EXAMPLE:—Assume a typical floor slab as shown in Fig. 10 with the spacing between columns of 18 feet each way, span of slabs 6 feet center to center of supporting beams, live load 250 lbs. per square foot, thickness of slab 4 inches, maximum working stress in the concrete 700 lbs. per square inch. For the bar design use \(\frac{1}{4}'' \) deformed bars at 12 inch centers for temperature reinforcement.

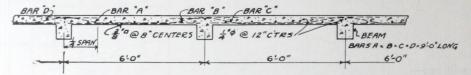


Fig. 11. Section through beams and slabs showing typical bar design for slabs.

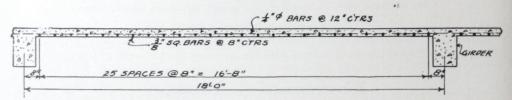


Fig. 12. Section through girders and slabs showing typical bar design or slabs.

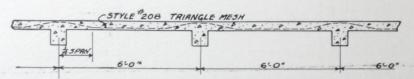


Fig. 13. Section through beam and slabs showing typical mesh design for slabs.

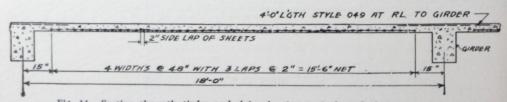


Fig. 14. Section through girder and slabs showing typical mesh design for slabs.

DESIGN No. 1.—Assume the same maximum working stress in the steel for bars and for mesh of 18,000 lbs. per square inch. Using standard designing formulas there will be required 0.21 square inches of steel per foot width of slab which can be supplied by 3/8-inch square deformed bars as detailed in Figs. 11 and 12 or by Triangle Mesh Reinforcement Style 208 as detailed in Figs. 13 and 14.

For each floor span between beams there will be required 26 \[^3/8\]-inch square deformed bars each 9 feet long bent as shown and 6 \[^1/4\]-inch round deformed bars each 19 feet long (allowing a minimum of 12 inches for end lap). For a typical 18' x 18' panel which contains 3 spans the total amount of bars required will be:—

26x3x9 = 702 Lin. ft. $\frac{3}{8}$ -inch square deformed bars, 702x.478 lbs. = 336 lbs. $\frac{3}{8}$ inch square deformed bars, and 6x3x19 = 342 Lin. ft. $\frac{1}{4}$ inch round deformed bars, 342x.167 lbs. = 57 lbs. $\frac{1}{4}$ -inch round deformed bars, making a total of 393 lbs. of bars.

For the 18'x18' panel there will be required 4 strips of Style 208 mesh each 4 feet wide and 18 feet long; also 8 strips of Style 049 each 4 feet wide and 4 feet long, making a total of 288 square feet of Style 208 and 32 square feet of Style 049. Style 208 weighs approximately 89 pounds per 100 square feet and Style 049, approximately 28 pounds per 100 square feet or a total weight of mesh for the 18'x18' panel of 274 pounds.

Design No. 2.—It is an established fact that wire mesh reinforcement can be safely stressed to a higher limit than bars. The standard working stress for bars is 16,000 lbs. per square inch instead of the 18,000 lbs. used in Design No. 1. With wire mesh this working stress can be safely increased to 20,000 lbs. per square inch. Using these stresses and other conditions the same as in Design No. 1, there will be required for bars a sectional area of 0.238 square inches per foot width which can be supplied by $\frac{3}{8}$ -inch square bars spaced 7 inches center to center. For the typical $\frac{18}{x18}$ panel there will be required 90 $\frac{3}{8}$ -inch square deformed bars, each 9 feet long, and $\frac{14}{4}$ -inch round deformed bars, each 19 feet long, or a total of $\frac{3}{8}$ -inch square bars and 57 lbs. of $\frac{1}{4}$ -inch round bars.

Using a maximum working stress for wire mesh of 20,000 lbs. per square inch there will be required a sectional area of 0.180 square inches per foot width which can be supplied by Triangle Mesh Reinforcement Style 180.

The total amount of mesh required for the 18'x18' panel is 288 square feet of Style 180 and 32 square feet of Style 049 or a total weight of mesh of 234 pounds.

New York Building Code Design—After an exhaustive series of tests the City of New York revised their building code covering reinforced concrete slab designs, as shown in the following article, effective March 14, 1916:

ARTICLE 17, Section 354, paragraph 4-e: "Strength of concrete slabs. In determining the safe carrying capacity of concrete slab floor fillings the gross load in pounds per square foot of floor surface shall not exceed the product of the depth in inches of the reinforcement below the top of the slab, by the cross-sectional area in square inches per foot of width of the tensional steel, divided by the square of the span in feet, all multiplied by the following co-efficients when cinder concrete is used: 14,000 if the reinforcement is not continuous over the supports, 18,000 if the reinforcement consists of rods or other shapes securely hooked over or attached to the supports, and 26,000 if the reinforcement consists of steel fabric continuous over the supports, and, when stone concrete is used, 16,000, 20,000 and 30,000, respectively." [Note:—This rule applies for spans 8 feet or less.]

Under this code Triangle Mesh Reinforcement style 107 will meet all the requirements as outlined in the example preceding and, therefore, still further economy may be secured.

COMPARATIVE COSTS:—In view of the fluctuating market and the necessarily differing prices at various points, no attempt will be made here to show the comparative costs, but by the aid of the quantities given above for a typical panel, the actual costs for the material f.o.b. cars at any desired point can be easily figured.

In addition to the cost of the material itself special attention must be given to the comparative costs for placing the steel in the forms ready for the concrete.

For average labor costs and continuous typical floor panels such as are found in factory or office buildings, use \$2.00 per ton for placing the fabric and \$15.00 to \$20.00 per ton for bending, placing and securing the bars.

High labor costs as well as restrictive labor conditions have a greater effect on the fabricating and placing costs for the bars than for the mesh. Under such conditions the saving by the use of fabric in long rolls is particularly noticeable.

Triangle Mesh Reinforced Slabs Supported by Steel Beams

Compare the costs in place of Triangle Mesh Fabric with any other form of concrete reinforcement and note the saving secured by its use. The comparative design shown on p. 9 will apply as well for steel as for all concrete framing.

The steel beams and steel columns should be wrapped with Triangle Mesh Reinforcement to prevent the concrete from breaking away from the framing and thereby exposing it to injury in case of fire.

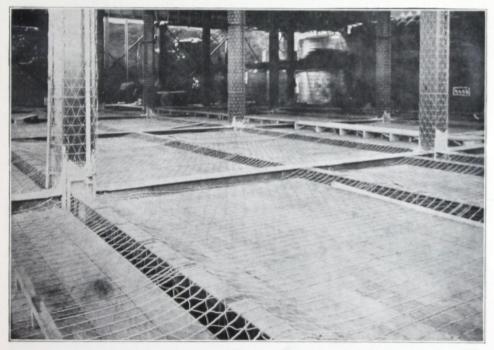


Fig. 15. Ballargeon Building, Seattle, Wash. Steel framing fireproofed and floor slabs reinforced with Triangle Mesh Reinforcement.

Concrete Joist Floors

For long spans and light loads a floor construction consisting of closely spaced concrete ribs and connecting concrete slabs will very often prove to be economical. Fig. 16 shows a typical section of such a floor. Here is a 2 inch or 3 inch slab having a clear span between the supporting ribs of 23 inches that must not only act as part of the compression portion of the beams and resist temperature stresses but in addition act as a support to the loads that may come upon the floor. The most efficient type of reinforcement to take care of these various

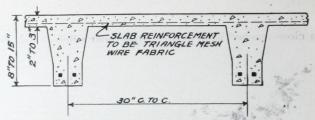


Fig. 16. Typical section of concrete joist floors.

stresses is a fabricated mesh made from cold drawn high elastic limit wire. At first these thin top slabs were built without any reinforcement. Such a construction invites disaster. The next step in the development of this type of floor consisted of adding small wire or rods placed at right angles to the ribs and spaced about 24 inches apart. Such a method possibly takes care of the temperature stresses but does not definitely insure the Tee-beam action assumed in the design and cannot possibly be considered as an efficient reinforcement to carry the live loads coming upon the floor. A thin slab approximately two feet square is left without any reinforcement. A wire mesh fabric with comparatively close spacing of members is the only logical reinforcement to use.

Reinforcement of Top Layer of Concrete Finished Floors

It is often desirable for construction reasons to leave the $\frac{3}{4}$ or 1 inch top dressing until after the main part of the slab is completely hardened. In this case the slab should be thoroughly cleaned before applying the final course and this course should be reinforced with Triangle Mesh Reinforcement Style 032 to prevent destructive cracks.

Reinforcement for Basement and Other Floors That Rest Upon the Ground

It is very seldom that such a concrete floor has better than filled ground for support and usually must carry heavy loads sometimes spread over a large area, sometimes concentrated on comparatively small space. Unless such a floor is reinforced it is liable to crack and then concrete is criticized unjustly. As conditions vary with each job it is impossible to make any definite suggestions regarding the proper weight of fabric to use but we can say that millions of square feet of Style 049 have been successfully used for this purpose. Where heavy loads and questionable fill-foundations occur it will be economy to use heavier mesh, such for instance as Style 153.

Wire Fabric Reinforcement for All Classes of Cement Gun Work

Gunite (trade name) is concrete applied by the Cement-gun process which produces probably the densest and therefore the most waterproof concrete available by present known methods. The density and waterproofness is undoubtedly due to the pressure continually applied to the green concrete, thereby eliminating as far as possible all voids or air pockets in the mixture. Although Gunite has high tensile and compressive strengths it still requires proper reinforcement to prevent cracks or in other words to produce a structurally strong finished product. Experience has taught the various Cement-gun companies that a close-meshed wire fabric is the most efficient and economical form of reinforcement to use for this purpose.

Gunite reinforced with Triangle Mesh Reinforcement has repaired successfully old crumbling bridge piers, tunnel linings and sea walls; old steel bridges, tanks and trusses; leaky reservoirs and irrigation ditches; even wooden structures that have been damaged by fire have been restored to usefulness by this method. And repair work is by no means the most important class of work produced. All kinds of new construction, especially those requiring waterproof

qualities with light resulting weight can be successfully executed.

Splendid work in Gunite has been done in the Construction of Concrete ships and barges and a specially interesting experiment has recently been made with this material in a reinforced concrete freight car. The reinforcing in these cases being bars with triangle wire fabric which distributes the stresses due to shock or unusual loading in an ideal manner.

2-inch mesh (2-inch spacing of cross wires known as "A" styles) is made for USE WITH THE CEMENT GUN or similar work. (See Table 4, page 30.)

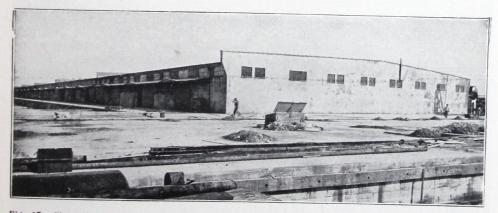


Fig. 17. Six miles of Gunite Walls on warehouses and piers at Army Supply Base. Norfolk, Va. Placed by Cement Gun Construction Co. of Chicago, using Style 26A Triangle Mesh Reinforcement.

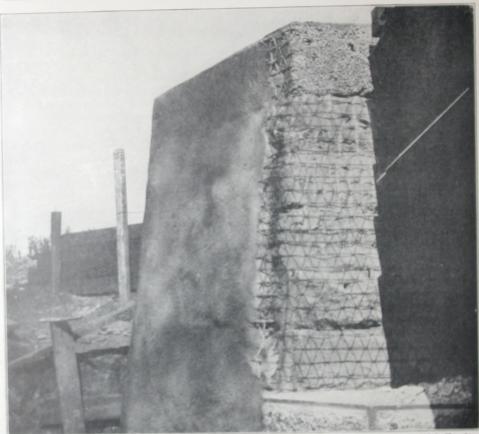


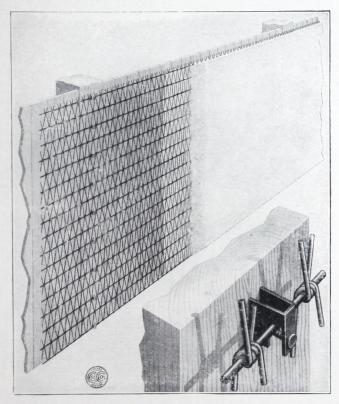
Fig. 18. Illustrating use of Triangle Mesh Reinforcement in restoring railroad bridge piers with Gunite.



Fig. 19. Showing Triangle Mesh Reinforcement in place, preparatory for the lining of a steel coal bunker with Gunite.



Fig 20.



Figs. 20, 21. Illustrating method used by the Cement Gun Construction Co. for holding Wire Mesh in place.

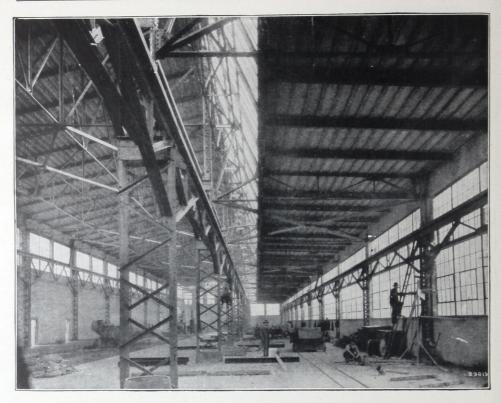


Fig. 22. American Steel & Wire Co.'s fabric is used exclusively in the manufacture of all concrete roof tile produced by the Federal Cement Tile Co. of Chicago.



Fig. 23. Showing one of the types of roof slab manufactured by the Federal Cement Tile Co. of Chicago.

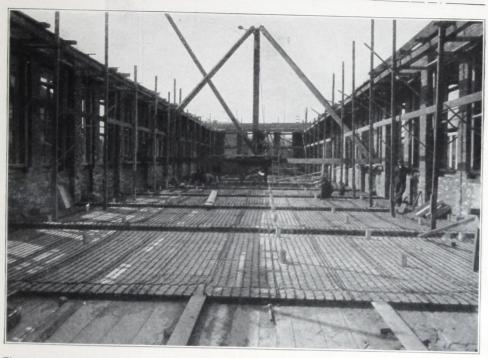


Fig. 24. No other form of reinforcement could have been laid as economically as was the Triangle Mesh Fabric in this floor construction.

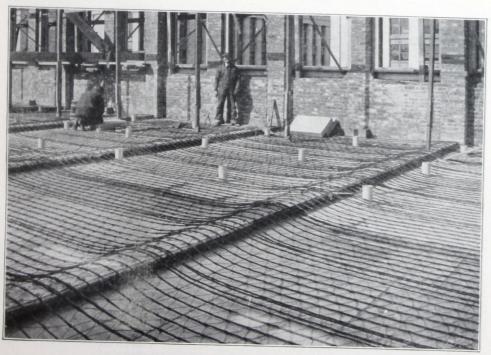


Fig. 25. Closer detailed view of the reinforcement shown in Fig. 24.

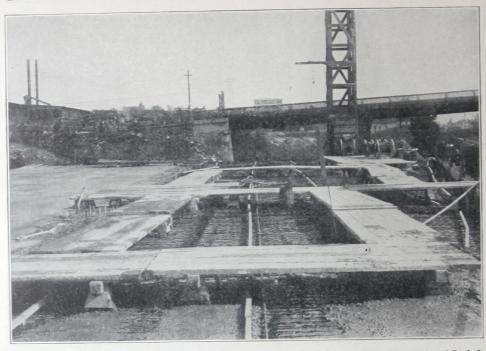


Fig. 26. Triangle Mesh Reinforcement as used with concrete supporting beams for the Norwood Sash & Door Co. Building, Norwood, Ohio.



Fig. 27. The low cost of installation is very often the deciding factor. Triangle Mesh Fabric assures minimum cost for placing in position ready for the concrete.

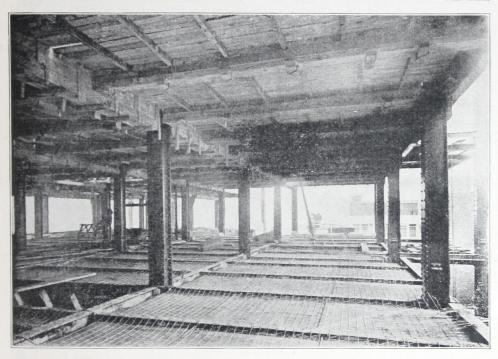


Fig. 28. Stern Brothers Building, 23rd Street, New York City. Architects, Maynicke & Franke; Contractors, Thompson-Starrett Co. 230,500 square feet Triangle Mesh used in the floors of this building.

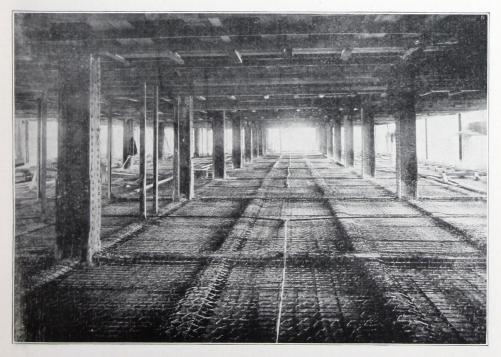


Fig. 29. For the floors, walls, foundations, columns and beams of this storage warehouse over 200,000 square feet of Triangle Mesh Fabric was used.

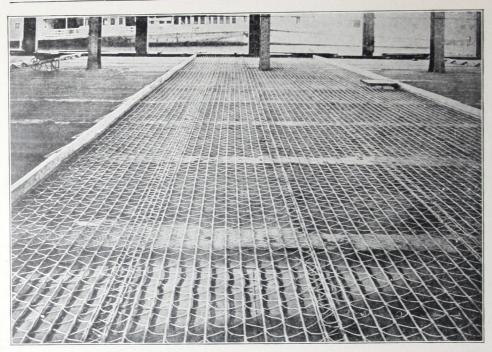


Fig. 30. Absolute minimum cost of installation resulted from the use of Triangle Mesh Reinforcement in Pier 42, North River, New York City. Notice how this heavy 3-strand fabric stays in correct position.

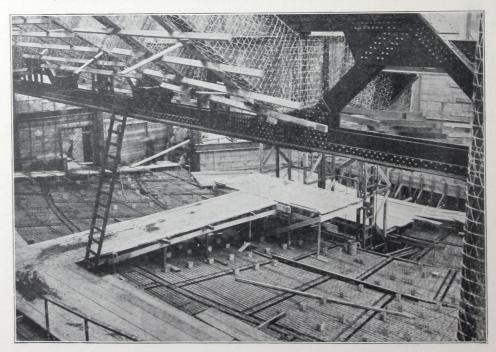


Fig. 31. Alcazar Theater, San Francisco, California. Triangle Mesh Fabric used throughout for reinforcing the floors and fireproofing all steel framing.

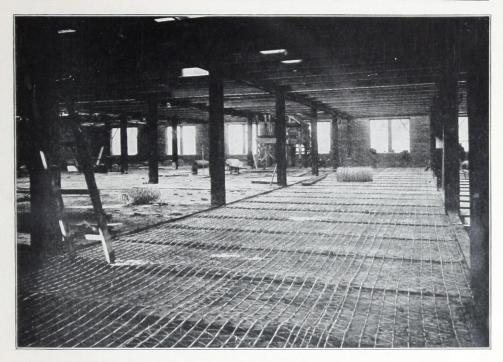


Fig. 32. Granger Building, Buffalo, N. Y.

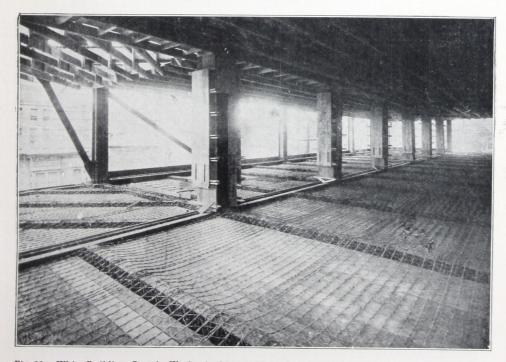


Fig. 33. White Building, Seattle, Wash. Architects, Howells & Stokes, New York & Seattle; Contractors Stone & Webster Engineering Corporation. Triangle Mesh Reinforcement is placed with the least possible amount of labor and stays where it is place.

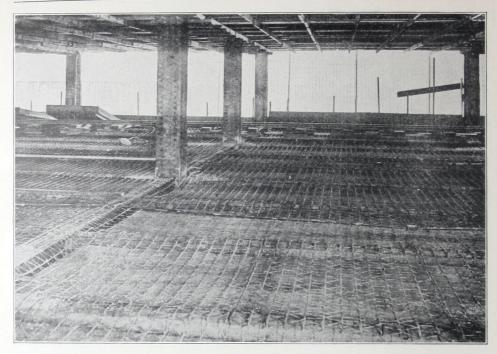


Fig. 34. Seattle Warehouse, Triangle Mesh Fabric used for the Floors, Walls, Foundations and for Fireproofing of the Steel Framing.

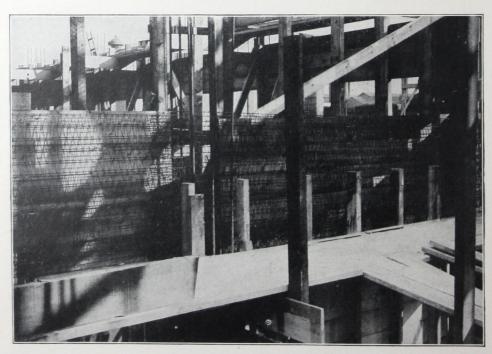


Fig. 35. An Economical and Efficient Wall Reinforcement. Sulphate of Iron Plant, National Tube Co., Lorain, Ohio.

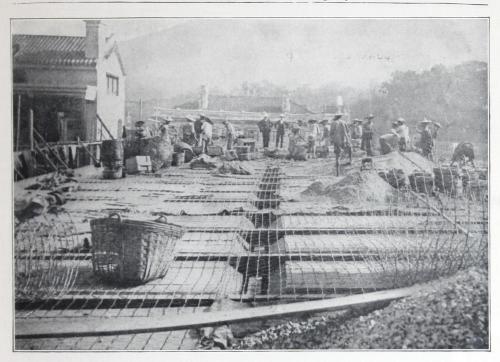


Fig. 36. Roof Construction of the Hong-Kong Hotel, Hong-Kong, China. Whether cheap or expensive labor is used, Triangle Mesh Fabric is the economical Reinforcement.



Fig. 37. Railway Station, Rangoon, Burma. Triangle Mesh Reinforcement is used in all parts of the World.

Industrial Pavements and Driveways

Save tractor power and maintenance cost by constructing reinforced concrete pavements in and around industrial plants. Practical experience as well as theoretical analysis shows the concrete pavement properly reinforced to be the most efficient as well as the most economical type that can be constructed for heavy as well as light industrial traffic.

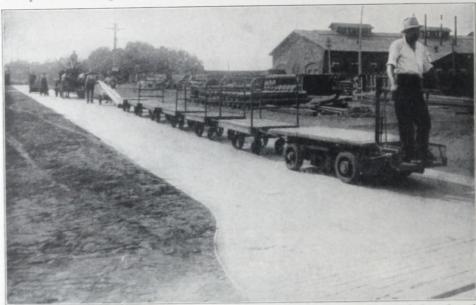


Fig. 38. Concrete driveways reinforced with Triangle Mesh Fabric have demonstrated the value of this class of construction for the distribution of shop materials at the Santa Fe Shops at Topeka, Kansas.

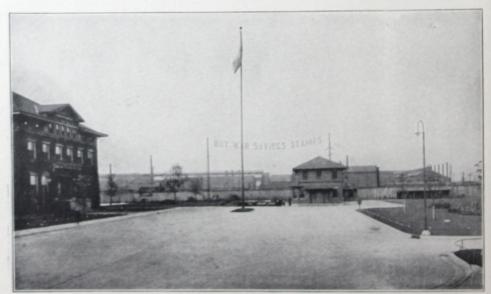


Fig. 39. Illinois Steel Co., Gary, Ind. This entrance driveway was constructed of Concrete properly reinforced with Triangle Mesh Fabric to prevent destructive cracks.

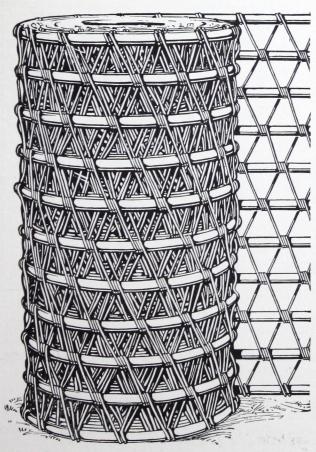
Description of Triangle Mesh Reinforcement

TRIANGLE MESH WOVEN WIRE REINFORCEMENT is made from cold drawn mild steel having a high breaking strength, the longitudinal or tension members are spaced 4 inches, the diagonal cross wires either 2, 4 or 8 inches.

For the light styles of fabric the longitudinals consist of one wire, for the medium styles two wires and for the heavy styles three wires stranded. The size of the wires is varied to obtain the desired cross sectional area of steel per foot of width. The reason for using stranded longitudinals for the heavy fabric is to reduce the stiffness of the finished product without affecting the tensile strength.

TRIANGLE MESH REINFORCEMENT is regularly made in standard rolls but can be furnished straightened and cut to lengths when required providing the tonnage is of a sufficient amount. As a general rule roll material can be more easily handled and installed in the work and should be preferred

by the user.



In Rolls

Table No. 1

Longitudinals Spaced 4 Inches
Cross Wires Number 14 Gauge Spaced 4 Inches

Number and Gauge of Wires, Areas per Foot Width and Weights per 100 Square Feet

Style Number	Number and Gauge of Wires each Longitudinal American Steel & Wire Company's Steel Wire Gauge	Sectional Area Longitudinals square inches per foot width	Total Effective Longitudinal Sectional Area square inches per foot width	Approximate Weight lbs. per 100 square feet
032	1—No. 12 gauge	.026	.032	22
040	1— " 11 "	.034	.040	25
049	1- ' 10 "	.043	.049	28
058	1- " 9 "	.052	.058	32
068	1- " 8 "	.062	.068	35
080	1- " 7 "	.074	.080	40
093	1- " 6 '	.087	.093	45
107	1- " 5 "	.101	.107	50
126	1- " 4 "	.120	.126	57
146	1— " 3 "	.140	.146	65
153	1— 1/4 inch	.147	. 153	68
168	1—No. 2 gauge	.162	.168	74
180	2- " 6 "	.174	.180	78
208	2- " 5 '	.202	.208	89
245	2- " 4 "	.239	. 245	103
267	3— " 6 "	.261	. 267	111
287	3- " 5.2"	.281	.287	119
309	3— " 5 "	.303	.309	128
336	3- " 4½"	.330	.336	138
365	3- " 4 "	.359	.365	149
395	3- " 31/2"	.389	.395	160

Length of Rolls: 150-foot, 200-foot and 300-foot.

Widths: Approximately 16-inch, 20-inch 24-inch, 28-inch, 32-inch, 36-inch, 40-inch, 44-inch, 48-inch, 52-inch and 56-inch.

Ultimate Tensile Strength 70,000 to 85,000 lbs. per sq. in.

NOTE.—Material may be furnished either plain or galvanized. Unless otherwise specified shipment will be made of material not galvanized.

Heavier Mesh than shown above can be furnished, maximum ½" round solid longitudinal Prices on request.

Table No. 2

Longitudinals Spaced 4 Inches Cross Wires Number 14 Gauge Spaced 8 Inches

Number and Gauge of Wires, Areas per Foot Width and Weights per 100 Square Feet

Style Number	Number and Gauge of Wires, each Longitudinal American Steel & Wire Company's Steel Wire Gauge	Effective Sectional Area of Cross Reinforcement square ins. per foot width	Effective Longitudinal Sectional Area square inches per foot width	Approximate Weight Ibs. per 100 square feet	
036P	1—No. 12 gauge	. 009	036	17	
044P	1- " 11 "	.009	044	20	
053P	1— " 10 "	.009	.053	24	
062P	1- " 9 "	.009	062	27	
072P	1- " 8 "	.009	072	31	
084P	1- " 7 "	.009	084	35	
097P	1- " 6 "	.009	.097	40	

Length of Rolls: 150-foot, 200-foot and 300-foot.

Widths: Approximately 16-inch, 20-inch, 24-inch, 28-inch, 32-inch, 36-inch, 40-inch, 44-inch, 48-inch, 52-inch and 56-inch.

Ultimate Tensile Strength 70,000 to 85,000 lbs. per sq. in.

Note. — Materials may be furnished either plain or galvanized. Unless otherwise specified, shipments will be made of material not galvanized.

Table No. 3

Longitudinals Spaced 4 Inches Cross Wires Number 12½ Gauge Spaced 8 Inches

Number and Gauge of Wires, Areas per Foot Width and Weights per 100 Square Feet

Style Number	Number and Gauge of Wires, each Longitudinal American Steel & Wire Company's Steel Wire Gauge	Effective Sectional Area of Cross Reinforcement square ins. per foot width	Effective Longitudinal Sectional Area square inches per foot width	Approximate Weight lbs. per 100 square feet
04.K	1—No. 12 gauge	.014	.041	21
049R	1- " 11 "	.014	.049	24
058R	1- " 10 "	.014	.058	28
067R	1- " 9 "	.014	.067	31
077R	1- " 8 "	.014	.077	35
089R	1- " 7 "	.014	.089	40
102R	1— " 6 "	.014	.102	44

Length of Rolls: 150-foot, 200-foot and 300-foot.

Widths: Approximately 16-inch, 20-inch, 24-inch, 28-inch, 32-inch, 36-inch, 40-inch, 44-inch, 48-inch, 52-inch and 56-inch.

Ultimate Tensile Strength 70,000 to 85,000 lbs. per sq. in.

Note.— Materials may be furnished either plain or galvanized. Unless otherwise specified, shipments will be made of material not galvanized.

Table No. 4

This Material is Used Principally for Cement Gun Work

Longitudinals Spaced 4 Inches Cross Wires Spaced 2 Inches

Style Number	Number of Wires Each Long	Gauge of Wire Each Long	Gauge of Cross Wires	Approximate Weight per 100 Square Feet
7-A	1	12	14	31
6-A	1	10	14	37
5-A	1	8	14	44
4-A	1	6	14	53
29-A	1	12	$12\frac{1}{2}$. 42
28 A	1	10	$12\frac{1}{2}$	48
27-A	1	8	$12\frac{1}{2}$	55
26-A	1	6	121/2	64

Length of Rolls: 150-foot, 200-foot and 300-foot.

Width: Approximately 16-inch, 20-inch, 24-inch, 28-inch, 32-inch, 36-inch, 40-inch, 44-inch, 48-inch, 52-inch and 56-inch.

Ultimate Tensile Strength 70,000 to 85,000 lbs. per sq. in.

Note.—Material may be furnished either plain or galvanized. Unless otherwise specified, shipments will be made of material not galvanized.

Table No. 5

Areas in Square Feet per Roll of Triangle
Mesh Reinforcement

Width of Roll	Square Feet of Reinforcement in Roll				
Inches	150-foot Roll	200-foot Roll	300-foot Rol		
16	200	267	400		
20	250	333	500		
24	300	400	600		
28	350	467	700		
32	400	533	800		
36	450	600	900		
40	500	667	1000		
44	550	733	1100		
48	600	800	1200		
52	650	867	1300		
56	700	933	1400		

For the Lighter Styles we recommend the use of 200 or 300-foot Rolls. The Heavy Styles are easier to handle in 150-foot Rolls.

Description of American Electrically Welded Fabric

American Electrically Welded Fabric is a square or rectangular mesh made from cold drawn steel wire electrically welded at the intersections of the transverse and longitudinal wires. Various combinations of spacings of wires can be furnished but the standard spacings for the longitudinals are 2, 3, 4 or 6 inches and for the transverse wires 8, 12 or 16 inches. For economical reasons it is advisable to select from the styles listed on the following pages.

The cross or transverse wires extend out one inch beyond the outside or

selvage longitudinal wires.

The weights given in the following tables are based on a width of fabric measured center to center of the outside longitudinals. Square footage is also based on a width exclusive of the overhang of the cross wires outside of the longitudinals.

It is regularly made in rolls for fabric having number 3 gauge or smaller longitudinal wires. If the longitudinals are larger than number 3 gauge, flat sheets only will be furnished. Any of the styles can be furnished straightened

and cut to lengths at an advance in price over that for rolls.

All widths are based on the distance center to center of the outside or selvage longitudinal wires. The maximum width of fabric depends on the spacing of the longitudinal wires, as follows: 96 inch maximum for 4 or 6 inch spacing, 84 inch maximum for 3 inch spacing and 60 inch maximum for 2 inch spacing.

American Electrically Welded Fabric combines the same high quality of material and service that has given Triangle Mesh Reinforcement its enviable reputation. When imbedded in concrete this fabric yields the maximum of its steel strength.

HOW TO ORDER

Specify the spacings of wires first, then the size or gauge. In each case mention first the longitudinals which are the wires running lengthwise of the roll or sheet.

EXAMPLE:—Assuming No. 6 gauge longitudinal wires spaced 4 inches and No. 10 gauge cross wires spaced 12 inches, the specifications should read: "American Welded Fabric 4 x 12 inch mesh, No. 6 x No. 10 wires."

The lengths and widths of rolls or sheets should be shown. If rolls are ordered and there is no preference as to widths or lengths, specify the total number of square feet and state any standard length and width of rolls will be satisfactory.

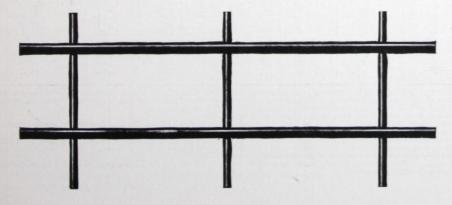


Table No. 6 Standard Styles American Electrically Welded Fabric

Weight in pounds per 100 sq. ft.	Sect. Area Square Inches Per Foot of Fabric		American Steel & Wire Company's Steel Wire Gauge No.		Spacing of Wires In Inches	
	Trans.	Longit.	Trans.	Longit.	Trans.	Longit.
138.9 119.4 103.6 88.5	.018 .015 .015 .013	. 377 . 325 . 280 . 239	7 8 8 9	1 2 3 4	16 16 16 16	2 2 2 2 2
82.6	.016	.216	8	2	13	3
74.6	.011	.202	10	5	16	2
72.0	.015	.187	8	3	16	3
64.7	.011	.174	10	6	16	2
61.4	.013	.159	9	4	16	3
56.1	.015	.140	8	3	16	4
51.8	.011	.135	10	5	16	3
47.9	.013	.120	9	4	16	4
45.1	.011	.116	10	6	16	3
40.4	.011	.101	10	5	16	4
38.1	.009	.098	11	7	16	3
35.2	.011	.087	10	6	16	4
31.7	.007	.082	12	8	16	3
29.7	.009	.074	11	7	16	4
25.5	.009	.062	12	8	12	4
21.8	.009	.052	12	9	12	4
18.6	.009	.043	12	10	12	4
12.6	.009	.026	12	12	12	4
48.4	.034	.101	5	5	12	4
41.6	.029	.087	6	6	12	4
35.4	.025	.074	7	7	12	4
29.6	.021	.062	8	8	12	4
65.3	.029	.148	6	0	12	6
59.4	.054	.108	2	2	12	6
51.0 43.8 37.0 31.8	.047 .040 .034 .029	. 093 . 080 . 067 . 058	3 4 5 6	3 4 5 6	12 12 12 12	6 6 6
27.0 11.1 57.8 48.8	.025 .013 .080 .067	.049 .017 .080 .067	7 12 4 5	7 12 4 5	12 8 6 6	6 6 6
42.0 35.7 30.0 25.0	.058 .049 .041 .035	. 058 . 049 . 041 . 035	6 7 8 9	6 7 8 9	6 6 6	6 6 6
20.7	.029	.029	10	10	6	6
85.3	.120	.120	4	4	4	4
61.9	.087	.087	6	6	4	4
44.1	.062	.062	8	8	4	4
60.3 36.8 27.7 21.2	.086 .052 .039 .030	. 086 . 052 . 039 . 030	10 12 13 14	10 12 13 14	2 2 2 2 2	2 2 2 2

Widths: Multiples of the spacing of longitudinal wire up to a maximum width which varies with the size and spacing of the longitudinals. Approximate maximums: 56-inch to 72-inch for 2-inch spacing, 84-inch to 96-inch for 3-inch or 4-inch spacing, and 96-inch to 120-inch for 6-inch spacing.

All widths measured center to center of selvage longitudinals.

The traverse wires extend 1 inch beyond the outside longitudinal wires. Square footage or square yardage will be figured exclusive of these projections. Extra charge made for widths narrower than 40 inches.

Length—Rolls:—Styles having longitudinals number 3 gauge or smaller made regularly in standard lengths 150 feet, 200 feet and 300 feet. Flat sheets can be furnished when desired. Styles having longitudinals larger than number 3 gauge made regularly in straighened and cut sheets only,

Weights:—All above weights are based on a width of 60 inches measured from center to center of the outside or selvage longitudinal wires.

Tables for Estimating Weight of American Electrically Welded Wire Fabric Table No. 7

Longitudinal Wires

Weights per 100 square feet Assuming Net Width of 60 inches Center to Center of Outside Wires

American Steel & Wire	-	S	pacing of Long	gitudinals, in I	nches	
Company's Steel Wire Gauge No.	2	3	4	6	8	12
0000	256.43	173.71	132.35	90.99	69.80	49.63
000	217.31	147.21	112.16	77.11	59.14	42.06
00	181.16	122.72	93.54	64.28	49.31	35.0€
0	155.37	105.25	80.19	55.13	42.29	30.07
1	132.43	89.71	68.35	46.99	36.05	25.63
2 3	113.96	77.20	58.82	40.44	31.02	22.06
3	98.21	66.53	50.69	34.85	26.73	19.03
4	83.95	56.87	43.33	29.79	22.85	16.2.
5	70.87	48.01	36.58	25.15	19.29	13.75
6	60.96	41.29	31.46	21.63	16.59	11.80
6 7	51.81	35.10	26.74	18.38	14.10	10.0
8	43.40	29.40	22.40	15.40	11.81	8.4
9	36.37	24.64	18.77	12.91	9.90	7.0
10	30.14	20.42	15.56	10.69	8.20	5.8
11	24.01	16.27	12.39	8.52	6.54	4.6
12	18.41	12.47	9.50	6.53	5.01	3.5
13	13.84	9.38	7.15	4.91	3.77	2.6
14	10.58	7.17	5.46	3.76	2.88	2.0

Table No. 8

Cross Wires

Weights per 100 square feet Assuming Net Width of 60 inches Center to Center of Outside Wires

American Steel & Wire		5	Spacing of Ca	ross Wire, in	Inches		
Company's Steel Wire Gauge No.	2	3	4	6	8	12	16
0000 000 00	256.43 217.31 181.16	170.95 144.87 120.78	128.22 108.66 90.58	85.48 72.44 60.39	64.11 54.33 45.29 38.84	42.74 36.22 30.19 25.90	32.05 27.16 22.65 19.42
0 1 2 3 4	155.37 132.43 113.96 98.21 83.95	103.58 88.29 75.97 65.47 55.97	77.69 66.22 56.99 49.10 41.97	51.79 44.14 37.99 32.74 27.98	33.11 28.49 24.55 20.99	22.07 18.99 16.37 13.99	16.55 14.24 12.28 10.49
5 6 7 8	70.87 60.96 51.81 43.40	47.24 40.64 34.54 28.93	35.43 30.48 25.90 21.70	23.62 20.32 17.27 14.47	17.72 15.24 12.95 10.85	11.81 10.16 8.63 7.23	8.86 7.62 6.48 5.43
9 10 11 12	36.37 30.14 24.01 18.41	24.25 20.09 16.01 12.27	18.18 15.07 12.01 9.20	12.12 10.05 8.00 6.14	9.09 7.53 6.00 4.60	6.06 5.02 4.00 3.07	4.58 3.77 3.00 2.30
13 14	13.84 10.58	9.23 7.06	6.92 5.29	4.61 3.53	3.46 2.65	2.31 1.76	1.73

The above weights are based on width of 60 inches measured from center to center of the outside or selvage longitudinal wires.

The weight of the cross or transverse wires includes the 1-inch projection or overhang beyond the outside longitudinal wires.

Table No. 9
Sectional Areas of American Welded Wire Fabric

Area in Square Inches per Foot of Width for Various Spacing of Wires

American Steel & Wire Company's	w	IRE		CENTE	R TO CE	NTER SP	ACING I	N INCHE	s
STEEL WIRE GAUGE NO.	Diam. Inches	Area Sq. Inches	2	3	4	6	8	12	16
000	. 3625	.10321	.619	.413	.310	.206	.155	.103	
00	. 3310	. 086049	.516	.344	.258	.172	.129	.086	
0	. 3065	.073782	.443	.295	.221	.148	.111	.074	.055
1	. 2830	.062902	. 377	.252	.189	.126	.094	.063	.047
2	. 2625	.054119	. 325	.216	.162	.108	.081	.054	. 041
3	. 2437	. 046645	. 280	.187	.140	.093	.070	.047	.035
4 5	. 2253	.039867	.239	.159	.120	.080	.060	.040	.030
5	. 2070	. 033654	. 202	. 135	.101	.067	.050	.034	.025
6	.1920	. 028953	.174	.116	.087	.058	.043	.029	.022
7	.1770	.024606	.148	.098	.074	.049	.037	.025	.018
8	. 1620	.020612	.124	.082	.062	.041	.031	.021	.015
9	. 1483	.017273	. 104	.069	.052	.035	.026	.017	.013
10	.1350	.014314	.086	.057	.043	.029	.021	.014	.011
11	.1205	.011404	. 068	.046	.034	.023	.017	.011	.009
12	. 1055	.0087417	. 052	.035	.026	.017	.013	.009	.007

Table No. 10
Square Feet per Roll of American Welded Reinforcement

Width of Rolls		FEET OF FABI PER ROLL	RIC	Width of	SQUA	RE FEET OF FA	BRIC
in Inches	150-foot Roll	200-foot Roll	300-foot Roll	Rolls in Inches	150-foot Roll	200-foot Roll	300-foo Roll
24	300	400	600	66	005	1100	
25	313	417	625	67	825	1100	1650
	-	211	020	68	838	1117	1675
26	325	433	650	69	850	1133	1700
27	338	450	675	70	863	1150	1725
28	350	467	700	10	875	1167	1750
29	363	483	725	71	000	1100	
30	375	500	750	72	888	1183	1775
		000	100	73	900	1200	1800
31	388	517	775	74	913 925	1217	1825
32	400	533	800	75		1233	1850
33	413	550	825	10	938	1250	1875
34	425	567	850	76	050	1007	
35	438	583	875	76 77	950	1267	1900
		000	010	78	963	1283	1925
36	450	600	900	79	975	1300	1950
37	463	617	925	80	988	1317	1975
38	475	633	950	80	1000	1333	2000
39	488	650	975	81	1012	1050	
40	500	667	1000	82	1013	1350	2025
			1000	83	1025 1038	1367	2050
41	513	683	1025	84		1383	2075
42	525	700	1050	85	1050	1400	2100
43	538	717	1075	00	1063	1417	2125
44	550	733	1100	86	1075	1400	0150
45	563	750	1125	87	1088	1433 1450	2150
10				88	1100	1467	2175
46	575	767	1150	89	1113	1483	2200
47	588	783	1175	90	1125	1500	2225
48	600	800	1200	00	1120	1500	2250
49	613	817	1225	91	1138	1517	2275
50	625	833	1250	92	1150	1533	2300
51	000			93	1163	1550	2325
52	638	850	1275	94	1175	1567	2350
53	650	867	1300	95	1188	1583	2375
54	663 675	883	1325		1100	1000	2010
55	688	900	1350	96	1200	1600	2400
00	088	917	1375	97	1213	1617	2425
56	700	000		98	1225	1633	2425
57	713	933	1400	99	1238	1650	2475
58	725	950 967	1425	100	1250	1667	2500
59	738	983	1450				2000
60	750	1000	1475	101	1263	1683	2525
	.00	1000	1500	102	1275	1700	2550
61	763	1017	1505				
62	775	1033	1525 1550				
63	788	1050	1575				
64	800	1067	1600				
65	813	1083	1625		The state of the s		

Explanation of Tables for Reinforced Concrete Slabs

Resisting Moment Tables, Pages 38 to 43

The following tables are based on the "straight line" formula. The ratio of the modulus of elasticity of steel to concrete is taken as fifteen. Values of resisting moments of slabs are given per foot of width for various maximum values for steel and concrete. Below and to the left of the heavy zigzag line, values of resisting moments are given as governed by the maximum allowable fiber stress in steel; the values above and to the right of this line are governed by maximum allowable fiber stress in concrete. The various values for maximum fiber stresses are thus given so that almost any specifications may be complied with.

The tables have been arranged in such a manner that a uniform reinforcement may be used and by increasing or decreasing the thickness of the slabs, spans of greater or lesser length than the average spans of the floor may be taken care of economically with the same reinforcement.

The second column gives the distance in inches from the center of the steel to the bottom of the slab. The third column gives the weight of the concrete slab per square foot of floor area, this weight being based on concrete weighing

144 pounds per cubic foot. It is considered good practice to use B. M. $=\frac{Wl^2}{12}$

when the slab is continuous over both supports; however, care must be taken to have the reinforcement near the top of the slab over supports in order to resist the negative bending moments at these points.

Examples of the use of these tables:

Given: A live load of 75 pounds per square foot; span of slab, 8 feet; floor, cement finish of slab, no plaster below; maximum allowable fiber stress in steel, 18,000 pounds per square inch; maximum allowable fiber stress in concrete, 700 pounds per square inch.

Look in the Table No. 19 giving minimum thickness of slabs for various spans. From this table we find for a load of 100 pounds per square foot and span of 8 feet, a minimum thickness of $3\frac{1}{2}$ inches is shown. It is better, however, to make this slab 4 inches thick.

The total dead load consists of the 4-inch slab, which will weigh

48 pounds per square foot.

Live loads 75 pounds per square foot.

Total 123 pounds per square foot.

For slabs continuous over both supports, figure the bending moment as follows.

Bending moment in foot pounds is equal to the total load per square foot multiplied by the span of the slab (in feet) squared and divided by twelve. Or expressed as a formula:

B. M. $=\frac{w^{1^2}}{12}$

In which

B. M. = Bending moment in foot pounds.

w = Total load in pounds per square foot.

I = Length of span in feet.

For this particular example

w= 123 (pounds per square foot).l= 8 (feet span of slab).

Then

B. M. =
$$\frac{123xS^2}{12}$$
 = 656 foot pounds.

Now it is necessary to find in Table No. 15 (corresponding to the given allowable stresses) the reinforcement for a 4-inch slab that will resist a bending moment of 656 foot pounds.

In this table we find a cross sectional area of steel of 0.15 square inch per foot width of slab will be needed to make a 4-inch slab capable of resisting a bending moment of this amount.

This area will be supplied with Triangle Mesh Reinforcement style number 153 as shown in the next to the last column in the table, page 28, or by American Electrically Welded Fabric 3" x 16" mesh No. 3 x No. 8 gauge wires as shown on page 32.

Slab Design Tables, See page 37

Table No. 37 shows at a glance the particular style number of Triangle Mesh Reinforcement to use with a given span and live load. The recommendations are based on the standard formula for reinforced concrete slabs using a bending moment of $WL^2 \div 12$, working stress in the steel of 18,000 lbs. per square inch and in the concrete of 700 lbs. per square inch. These assumptions will agree with the majority of building codes. Experience indicates they are decidedly conservative for short spans at least.

If the spans are continuous over one support only, the thickness of slabs as shown in Table No. 12 may be increased by $\frac{1}{2}$ inch or 1-inch as the table indicates or the reinforcement increased in accordance with the substitutions shown in Table No. 11. Using either of these methods is equivalent to assuming a bending moment of $WL^2 \div 10$ which agrees with the standard formulas.

Table No. 11

Specified	Styles	036P	044P	053P	062P	072P	084P	097P	107	126	146
Substitute	Styles	044P	053P	062P	072P	084P	097P	107	126	146	168
Specified Substitute	Styles	153		180	208	245	267	287 309			

Note:—Specified styles are those shown in table No. 12 and are based on a bending moment of WL^2+12 . Substitute Styles are the ones to use in place of the corresponding Specified Styles shown in table No. 12 when a bending moment of WL^2+10 is assumed unless the slab thickness is increased as indicated in table No. 12, in which case the Specified Styles apply.

After hundreds of tests of actual slabs made by the Testing Laboratory of Columbia University, the City of New York prepared a code covering slabs having spans of 8 feet or less and decided on the use of the empirical formula as shown on page 11 of this booklet. This formula is equivalent to assuming a working stress in the steel in the standard formula for reinforced concrete design of approximately 32,000 lbs. per square inch. Millions of square feet of reinforced concrete floors have been built in accordance with this New York code.

Style Number of Triangle Mesh Reinforcement and Thickness of Slab for Given Spans and Loads

Center of Steel 34 inch above bottom of slab. Bending Moment = WL² ÷ 12.

If the spans are continuous over one support only the reinforcement may be increased as shown in Table No. 11, or the thickness of slab may be Reinforcement in each case is sufficient for the live load shown plus the weight of the concrete slab. Any dead load of the floor exclusive of the concrete slab should be considered as part of the live load shown here.

Maximum Stresses: Steel = 18,000 Pounds, Concrete = 700 Pounds

increased by ½ inch for values above and to the left and by 1-inch for values below and to the right of the heavy ziz-zag line in the table below.

Load							NI TO TO															
are	4'-0"	4'-6"	5'-0"		, 0-,9	, 9-,9	1,0-,2	1,9-,2	8,-0"	8,-6"	0-/6	0,6	10,-0"	10,-6"	11,-0"	11,-6"	12'-0"	12'-6"	13'-0"	13'-6"	14'-0"	14'-6
30#	3" 036P	3" 036P	3" 044P	3" 053P	3" 062P	3" 084P	3" 097P	3" 107	3" 126	3"	3"	31/2"	31/2"	4"	4" 180	4" 208	4½" 208	41/2" 245	41/2"	41/2" 267	5" 267	5" 287
#0#	3" 036P	3" 044P	3" 053P	3" 062P	3" 072P	3" 097P	3" 107	3" 126	3"	3"	3½" 153	31/2"	31/2"	4" 180	4" 208	4" 245	41/2" 245	41/2" 267	41/2""	5" 287	5" 309	51/2"
20#	4" 036P	4" 036P	4" 044P	4" 053P	4" 072P	4" 084P	4" 097P	4" 107	4"	4" 146	4"	4"	4" 180	4" 208	4" 245	4½" 245	41/2" 267	5" 267	5" 287	5" 309	51/2" 309	51/2" 336
#09	4" 036P	4" 044P	4" 053P	4" 062P	4" 072P	4" 097P	4" 107	4"	4" 146	4" 153	4"	4" 180	4" 208	4" 245	41/2" 245	41/2" 267	5" 267	5" 287	5" 309			
#08	4" 036P	4" 053P	4" 062P	4" 072P	4" 097P	4" 107	4"	4" 146	4"	180	4" 208	41/2" 208	41/2" 245	41/2" 245	5" 267	5" 287	5" 309	51/2" 309				
#001	4" 044P	4" 053P	4" 072P	4" 084P	4" 107	4" 126	4" 146	4"	4" 180	4, 208	4" 245	4½" 245	41/2 267	5" 267	5" 287	51/2"	51/2" 336	6" 336			7	
125#	4" 053P	4" 062P	4" 084P	4" 097P	4" 126	4" 146	4" 168	4" 180	4" 208	41/2" 208	41/2" 245	5" 245	5" 287	5" 309	51/2"	608	61/2" 336					
150#	4" 062P	4" 072P	4" 097P	4" 126	4" 146	4"	4" 180	4" 208	4½" 208	41/2" 245	5" 267	5" 287	5½" 287	5½" 336	6"		. 50					
175#	4" 072P	4" 084P	4" 107	4" 126	4" 153	180	4" 208	4½" 208	41/2" 245	5" 267	5" 287	5½" 237	51/2"	6" 336								
200#	4" 072P	4" 097P	4" 126	4" 146	4" 168	4,"	41/2 " 208	41/2" 245	5" 245	5" 287	51/2"	51,2 "	6" 336									
250#	4" 097P	4" 126	4 " 146	4" 168	4, 208	4½" 208	41/2" 267	5" 267	5"	51/2"	908	61/2"										
300#	4" 107	4" 146	4" 168	4" 208	41/2"	41/2" 267	5" 267	5"	51/2"	6,												

For sectional areas, weights, etc., of Triangle Mesh Wire Reinforcement, see pages 28-29-30.

For sectional areas, weights, etc., of American Electrically Welded Fabric, see pages 32-33-34.

American Electrically Welded Fabric may be substituted for the Triangle Mesh specified provided equal sectional areas per foot are used.

Fable No. 13

Area of Steel Required per Foot of Width for a Maximum Resisting Moment of Slab of Given Thickness

Corresponding safe bending moment due to applied load and weight of floor:

The maximum allowable fiber stress in the steel governs the values of resisting moments given below and to the left of the heavy zigzag line; the maximum allowable fiber stress in the concrete governs the values above and to the right of this line.

Maximum Stresses: Steel = 18,000 Pounds, Concrete = 600 Pounds

Concrete 1:2%:5

152 1300 1522 1668 1887 2320 2749 3173 3664 4061 4207 4341 4465 4581 4689 4806 4900 5076 5233 1.00 1514 1697 1879 2059 2598 3041 3569 4004 4524 5014 5164 5333 5462 5609 5723 5852 6069 6266 5741 5910 6099 6243 6381 6511 6773 6989 7490 7747 243 1414 1586 1840 2009 2511 3006 3416 3906 4386 4715 4880 5032 5174 5284 5409 5526 5724 06 7234 8 6745 6917 7048 $1050 | 1240 | 1366 | 1554 | 1741 | 2110 | 2587 | 2959 | \overline{3318} | 3566 | 3703 | 3810 | 3927 | 4034 | 4120 | 4215 | 3120 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215 | 3215$ 22 (027 | 1144 | 1318 | 1491 | 1663 | 2059 | 2452 | 2840 | 3209 | 3324 | 3463 | 3574 | 3663 | 3759 | 3848 | 3931 |2 Cross Sectional Area in Square Inches of Steel Reinforcement per Foot of Width 913 1059 1206 1349 1493 1872 2200 2571 2775 2882 2990 3078 3169 3253 65 1115 1234 1352 1702 2010 2256 2370 2461 2544 2628 2698 5700 6252 6557 09 Moments of Resistance in Foot Pounds per Foot of Width 999 1095 1222 1504 1805 1900 1984 2060 2136 2198 55 965 1090 1336 1477 1561 1627 1693 1747 618 1827 2034 2240 2753 3259 3761 4358 4852 5342 20 914 1001 1238 1333 1402 1463 1517 2413 2993 3451 4020 4584 5144 .45 986 1048 1103 1150 40 35 792 .30 534 .25 325 496 694 865 200 2180 315 480 784 999 .18 585 826 302 865 459 703 .16 515 926 290 716 422 764 871 621 .14 368 663 835 .12 765 859 878 515 999 644 851 454 456 512 550 189 250 301 370 424 828 352 267 309 379 146 185 90. 28 154 081 .04 97 Weight of Slab per Square Foot Pounds 102 108 428 54 60 60 72 78 90 96 of Steel to Bottom Center of Slab XXXX 14 Thickness of Slab Inches Total 8 4 4 8 6 %

For examples showing use of tables, see pages 35-36. For sectional areas, weights, etc., of Triangle Mesh Wire Reinforcement, see pages 28-29-30. For sectional areas, weights, etc., of American Electrically Welded Fabric, see pages 32-33-34. Nore-

Area of Steel Required per Foot of Width for a Maximum Resisting Moment of Slab of Given Thickness

Corresponding safe bending moment due to applied load and weight of floor.

The maximum allowable fiber stress in the steel governs the values of resisting moments given below and to the left of the heavy zigzag line; the maximum allowable fiber stress in the concrete governs the values above and to the right of this line.

Maximum Stresses: Steel=18,000 Pounds, Concrete=650 Pounds Con-

Concrete 1:2%:5 carefully graded

		MIT-1-LA							IMI	ments	10	Sistant	-	100	Outra	Moments of Resistance in Foot Founds per root								1
	Center of Steel to	of Slab					O	ross S	ections	1 Area	in S	quare	Inches	of St	eel R	einforc	ement	per F	oot of	Cross Sectional Area in Square Inches of Steel Reinforcement per Foot of Width				
of Slab Bot Inches of S	ttom	Per Square Foot Pounds	.04	90.	80.	01.	.12	.14	.16	.18	.20	.25	.30	355	.40	.45	. 50	.55	09.	:3:	02	7.5	06. 08	0 1.00
1/2	3%	, 30	97	146	189	237	279	314	328	841	853	377												
,	4 %	36	128	185	250	305	368	422	476	520	537	578	611								_			
2%	34	43	154	858	301	373	444	515	585	999	734	810	828	006							-			
	164	48	180	267	370	454	538	621	708	784	865	1068	1136	865 1068 1136 1194 1246	1246								_	
11/2	34	54	216	309	424	515	627	716	826		1001	1238	1444	914 1001 1238 1444 1519 1585 1644	1585	1644							_	
	1	09		352	456	999	663	764		965	1090	1336	1578	1691	1764	965 1090 1336 1578 1691 1764 1835 1893	1893							
51/2	1	99		379	513	644	741		666	1095	1222	1504	1814	2058	2150	999 1095 1222 1504 1814 2058 2150 2232 2314 2381	2314	2381						
3	1	73			550	714	835		$956\ 1115\ 1234\ 1352\ 1702\ 2010\ 2315\ 2568\ 2666\ 2756\ 2848\ 2922$	1234	1352	1702	2010	2315	2568	5666	2756	2848	2922					
21%	1	78			616	765		1059	913 1059 1206 1349 1443 1872 2200 2571 2938 3123 3240 3334 3431 3525 3240	1349	1493	1872	2200	2571	2938	3123	3240	3334	3431	3525				_
	-	84				851		1144	1318	1491	1663	2059	2452	2840	3225	3551	3752	3872	3968	4072	$1027111441318149116632059245228403225\overline{3551}\overline{3752}38723968407241694259$	528		
71%	11/4	06				829	1050	1240	1366	1554	1741	2110	2537	2959	3318	3735	4012	4128	4255	4371	$1050\ 1240\ 1366\ 1554\ 1741\ 2110\ 2537\ 2959\ 3318\ 3735\ 4012\ 4128\ 4255\ 4371\ 4464\ 4566$	9999		-
. 80	11/4	96					1152	1300	1522	1668	1887	2320	2749	3173	3664	4081	4494	4703	4837	4963	1152 1300 1522 1668 1887 2320 2749 3173 3664 4081 4494 4703 4837 4963 5080 5207 5309 5498 5668	207 55	08 54	98 26
81%	11/4	102					1243	1414	1586	1840	2009	2511	3006	3416	3906	4386	4789	5264	5452	5605	243 1414 1586 1840 2009 2511 3006 3416 3906 4386 4789 5264 5452 5605 5725 5860 5987 6201 6410	860 55	87 62	01 64
	11/2	108						1514	1697	1879	2059	2598	3041	3569	4004	4524	5038	5464	5777	5918	[514] $[697]$ $[1879]$ $[2059]$ $[2598]$ $[3041]$ $[3569]$ $[4004]$ $[4524]$ $[5038]$ $[5464]$ $[5777]$ $[5918]$ $[6077]$ $[6200]$ $[6340]$ $[6776]$ $[6788]$	200 65	340 65	76 67
	11/2	114						1618	1827	2034	2240	2753	3259	3761	4358	4852	5342	5830	6313	2099	1618 1827 2034 2240 2753 3259 3761 4358 4852 5342 5830 6313 6607 6764 6914 7055 7338 7571	914 70	55 73	38 75
	11/	190							1944	9180	9413	2993	3451	4020	4584	5144	5700	6252	6810	1944 2180 2413 2993 3451 4020 4584 5144 5700 6252 6810 7307 7494 7636 7836 8114 8393 8888	7494 7	929	336 81	14 83

Nore—For sectional areas, weights, etc., of Triangle Mesh Wire Reinforcement, see pages 28-29-30. For examples showing use of tables, see page 35-36. For sectional areas, weights, etc., of American Electrically Welded Fabric, see pages 32-33-34.

Area of Steel Required per Foot of Width for a Maximum Resisting Moment of Slab of Given Thickness

Corresponding safe bending moment due to applied load and weight of floor:

The maximum allowable fiber stress in the steel governs the values of resisting moments given below and to the left of the heavy zigzag line; the maximum allowable fiber stress in the concrete governs the values above and to the right of this line.

Maximum Stresses: Steel = 18,000 Pounds, Concrete = 700 Pounds

Concrete 1:2:4

	1	1.00												104	903	7310	154	038
		90												950 6	678 6	081 7	8 206	$1944 \ 2180 \ 2418 \ 2998 \ 3451 \ 4020 \ 4584 \ 5144 \ 5700 \ 6252 \ 6810 \ 7345 \ 7859 \ 8224 \ 8440 \ 8739 \ 9038$
		08:												717 5	447 6	828	597 7	4408
		.75										989	913	807 5	311 6	8 229	446 7	224 8
		.70										190 41	302 49	120 2	9991	544 6	284 7	8688
	/idth	.65									96.	886 44	104 48	345 54	35 6	373 6	300 73	345 7
	t of W	09								47	98 37	74 48	79 47	10 55	38 60	72 65	313 62	310 78
Vidth	r Foo	.55							92	66 31	91 36	70 42	42 45	05 52	64 57	64 59	30 65	25 68
Moments of Resistance in Foot Pounds per Foot of Width	ent pe	.50						38	999 1095 1222 1504 1814 2089 2316 2404 2492 2565	68 30	89 35	84 41	43 44	94 49	89 52	38 54	42 58	00
er Foo	orceme						0.2	965 1090 1336 1578 1821 1899 1976 2038	04 24	71 29	57 34	51 39	35 41	81 44	86 47	24 50	52 53	44 57
nds be	Reinf	.45				23	717	9 19	6 24(4 28	8 32	5 35	8 37	4 40	6 43	45	8 48	34 51
Pour	iteel	.40			6	3 134	3 170	188	9231	5 265	1 293	0 322	9 331	3 366	6 390	9 400	1 435	0 458
Foot	s of s				696	1286	1630	185	508	231	257	284	292	317	3 341	1356	9376	1 402
nce in	Inche	.30		658	924	1223	1493	1578	1814	2010	2200	245	2537	2748	3006	304	325	342
esista	quare	.25	406	623	872	865 1074 1223 1286 1342	1238	1336	1504	1702	1872	2059	2110	2320	2511	2598	2758	3888
s of R	in S	.20	379				914 1001 1238 1493 1636 1707 1770	1090	1222	1352	1493	1663	1741	1887	2009	2059	2240	2418
oment	1 Area	.18	367	537	999	784			1095	1234	1349	1491	1554	1668	1840	1879	2034	2180
Me	ctiona	.16	353	476	585	703	826	865		$956\ 1115\ 1234\ 1352\ 1702\ 2010\ 2315\ 2654\ 2871\ 2968\ 3066\ 3147$	1206	1318	1366	1522	1586	514 1697 1879 2059 2598 3041 3569 4004 4524 5038 5464 5972 6373 6544 6677 6828 7081	(618)1827[2034]2240[2753]3259[3761]4358[4852]5342[6830]6313[6890]7284[7446]7597[7902]8154[7818]784[7846]7597[7902]8154[7818]7[7902]8154[7902	1944
	Cross Sectional Area in Square Inches of Steel Reinforcement per Foot of Width	.14	325	422	515	621	716	764	871		913 1059 1206 1349 1493 1872 2200 2571 2938 3257 3489 3591 3698 3796	$1027 1144 1318 1491 1663 2059 2452 2840 3225 3551 \overline{3984} 4170 4274 4386 4490 4586 \overline{1100} 1000 \overline{1100} 110$	1050 1240 1366 1554 1741 2110 2537 2959 3318 3735 4143 4442 4579 4704 4802 4913	1152 1300 1522 1668 1887 2320 2749 3173 3664 4081 4494 4905 5210 5345 5470 5607 5717 5920 6104	[243] 1414 1586 1840 2009 2511 3006 3416 3906 4386 4789 5264 5738 6035 6166 6311 6447 6678 6903	1514	1618	
	C	.12	279	368	444	538	627	663	741	835	913	1027	1050	1152	1243			
		.10	237	305	373	454	515	260	644	714	765	851	859					
		80.	189	250	301	370	424	456	512	550	616							
		90.	146	185	858	267	309	352	379									
		10.	97	128	154	180	216											
Weight	o	Foot	30	36	42	48	54	09	99	72	78	84	06	96	102	108	114	120
	of Steel to	of Slab	3%	124	*	×	*	1	1	1	1	1	174	1,4	1,4	1 1/2	1 1/2	1 1/2
F	Thickness	Inches	21/2	89	3 1/2	4	4 1/2	5	5 1/2	9	61/2	7	77%	8	81/2	6	9 1/2	10

Nore—For sectional areas, weights, etc., of Triangle Mesh Wire Reinforcement, see pages 28-29-30. For examples showing use of tables, see page 35-36. For sectional areas, weights, etc., of American Electrically Welded Fabric, see pages 32-33-34.

Fable No. 16

Area of Steel Required per Foot of Width for a Maximum Resisting Moment of Slab of Given Thickness

Corresponding safe bending moment due to applied load and weight of floor:

The maximum allowable fiber stress in the steel governs the values of resisting moments given below and to the left of the heavy zigzag line; the maximum allowable fiber stress in the concrete governs the values above and to the right of this line.

Concrete 1:21/2:5

Maximum Stresses: Steel = 20,000 Pounds, Concrete = 600 Pounds

4207 4341 4465 4581 4689 4806 4900 5075 5232 1682 |1886|2087|2288|2887|3378|3966|4449|4817|5014|5164|5333|5462|5609|5723|5862|6069|62661.00 1168 | 1379 | 1519 | 1728 | 1935 | 2346 | 2821 | 3272 | 3415 | 3566 | 3703 | 3810 | 3927 | 4034 | 4120 | 4215 | 3212 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 | 4215 |1142[1271]1464[1657]1848[2289]2724[3061]3209[3324]3463[3574]3663[3759]3848[3931]142[324]3463[3574]3663[3759]3848[3931]142[324]3463[3574]3663[3759]3848[3931]142[367]364[367] $2160 \left[2422 \left[2680 \right] 3325 \left[3834 \right] 4467 \left[5094 \right] \overline{5715} \left[6143 \right] 6359 \left| 6557 \right| 6745 \left| 6917 \right| 6$ Cross Sectional Area in Square Inches of Steel Reinforcement per Foot of Width 1014[1177[1338]1499[1659]2080[2446]2645[2775]2882[2990]3078]3169]3253Moments of Resistance in Foot Pounds per Foot of Width 3 1211 1389 1477 1561 1627 1693 1747 $280|1445|1690|1853|2097|2577|3054|\overline{3525}|\overline{3902}|4061|$ 918 1015 1112 1245 1333 1402 1463 1517 .45 1048 1103 1150 .85 .20 .18 .16 .14 .12 Weight of Slab per Square Foot Pounds 84 84 60 66 72 of Steel to Bottom of Slab Center Thickness of Slab 6 5 1/2 Total

For sectional areas, weights, etc., of Triangle Mesh Wire Reinforc, ment, see pages 28-29-30. For examples showing use of tables, see page 35-36. For sectional areas, weights, etc., of American Electrically Welded Fabric, see pages 32-33-34. NOTE

Area of Steel Required per Foot of Width for a Maximum Resisting Moment of Slab of Given Thickness Corresponding safe bending moment due to applied load and weight of floor. The maximum allowable fiber stress in the steel governs the values of resisting moments given below and to the left of the heavy zigzag lines; the maximum allowable fiber stress in the concrete governs the values above and to the right of this line.

1.00 $280 \left[1445 \left[1690 \right] 1853 \left[2097 \left[2577 \right] 3054 \left[3525 \right] 4071 \right] 4400 \left[4558 \right] 4703 \left[4837 \right] 4963 \left[5080 \left[5207 \right] 5309 \left[5498 \right] 5658 \right] 4888 \left[3680 \left[3680 \right] 5889 \left[3680 \left[3680 \right] 5888 \right] 4888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \right] 4888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \right] 4888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \right] 4888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \right] 4888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \right] 4888 \left[3680 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \right] 5888 \left[3680 \left[3680 \right] 5888 \left[3680 \right]$ 383 | 1571 | 1762 | 2045 | 2232 | 2789 | 3341 | 3796 | 4338 | 4874 | 5108 | 5286 | 5452 | 5605 | 5725 | 5860 | 5987 | 6201 | 6410 | 5881 | 5881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 6881 | 61682 | 1886 | 2087 | 2288 | 2887 | 3378 | 3966 | 4449 | 5026 | 5432 | 5594 | 5777 | 5918 | 6077 | 6200 | 6340 | 6576 | 6788 2160[2422[2680]3325[3834]4467[5094[5715[6333]6890]7104[7307]7494[7636]7836[8114]8893] $.798 |\, 2030 |\, 2260 |\, 2490 |\, 3058 |\, 3620 |\, 4179 |\, 4842 |\, 5391 |\, 5936 |\, 66220 |\, 6403 |\, 6607 |\, 6764 |\, 6914 |\, 7055 |\, 7338 |\, 7571 |\, 7086 |\, 7886 |\, 7887 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 7888 |\, 788$ Concrete | 1:254 | Concrete | 1:2%:5 carefully graded 8 89 1142 1271 1464 1657 1848 2289 2724 3156 3476 3601 3752 3872 3968 4072 4169 4259 $1168 \, | 1379 | 1519 | 1728 | 1935 | 2346 | 2821 | 3290 | \overline{3688} | \overline{3863} | 4012 | 4128 | 4255 | 4371 | 4464 | 4566 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 | 4268 |$ 22 2. Cross Sectional Area in Square Inches of Steel Reinforcement per Foot of Width 1014 | 1177 | 1338 | 1499 | 1659 | 2080 | 2445 | 2857 | 3008 | 3123 | 3240 | 3534 | 3431 | 3525 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 | 3240 |.65 1063 1239 1371 1502 1892 2234 2444 2568 2666 2756 2848 2922 09 Moments of Resistance in Foot Pounds per Foot of Width 22 968 1110 1216 1358 1671 1956 2058 2150 2232 2314 2381 961 1073 1211 1484 1601 1691 1764 1835 1893 3 918 1015 1112 1349 1444 1519 1585 1644 .45 Maximum Stresses: Steel=20,000 Pounds, Concrete=650 Pounds 961 1068 1136 1194 1246 40 006 .85 810 858 578 611 .30 35 752 537 353 .20 520 726 341 .18 328 498 650 781 .16 572 314 469 069 795 849 14 928 1 409 697 736 297 597 494 824 .13 946 850 263 415 505 572 655 .10 210 569 334 508 471 162 206 253 297 344 90. 171 200 108 143 240 04 per Square Weight of Slab Foot Pounds 43 48 54 66 60 72 78 84 90 96 801 202 of Steel to Bottom of Slab Center XXXX 17 1 1/2 Thickness of Slab 5 1/2 Total Inches

For sectional areas, weights, etc., of Triangle Mesh Wire Reinforcement, see pages 28-29-30. For examples showing use of tables, see page 35-36. For sectional areas, weights, etc., of American Electrically Welded Fabric see pages 32-33-34. NOTE

Table No. 18

Area of Steel Required per Foot of Width for a Maximum Resisting Moment of Slab of Given Thickness

Corresponding safe bending moment due to applied load and weight of floor:

Total Thickness of Slab Inches

The maximum allowable fiber stress in the steel governs the values of resisting moments given below and to the left of the heavy zigzag line; the maximum allowable fiber stress in the concrete governs the values above and to the right of this line.

		,		,											_	~	_	-	~
			1 00												610	9069	7310	815	903
2,4			06.												5920	8299	7081	7902	8238
Concrete, 1:2:4			98.												5717	6447	6828	7597	8440
neret			.75										4586	4913	2607	6311	2299	7446	8224
Co			.70										4490	4802	5470	9919	6544	7284	8071
		Width	.65									3796	4386	4704	5345	6035	8373	7115	1869
	th	ot of	09.								3147	3698	4274	4579	5210	5871	8222	6895	7555
	Widt	per Fo	35.							2565	9908	3591	4170	1442	2065	2693	3024	3477	3946
Maximum Stresses: Steel = 20,000 Pounds, Concrete = 700 Pounds	Moments of Resistance in Foot Pounds per Foot of Width	Cross Sectional Area in Square Inches of Steel Reinforcement per Foot of Width	.50						2038	9681110 1216 1358 1671 2016 2217 2316 2404 2492 2565 2667 266	928 1063 1239 1371 1562 1892 2234 2572 2765 2871 2968 3066 3147	850 1014 1177 1338 1499 1659 2080 2445 2857 3240 3363 3489 3591 3698 3796	1142 1271 1464 1657 1848 12289 12724 13156 13583 13878 4040 4170 4274 4386 4490 4586 12	$1168 1379 1519 1728 1935 2346 2821 3290 3688 \overline{4150} \underline{4320} \underline{4442} \underline{4579} \underline{4704} \underline{4802} \underline{4913} \underline$	280 1445 1690 1853 2097 2577 3054 3525 4071 4534 4908 5065 5210 5345 5470 5607 5717 5920 6104	383 1571 1762 2045 2232 2789 3341 3796 4338 4874 5320 5693 5871 6035 6166 6311 6447 6678 6903	1682 1886 2087 2288 2887 3378 3966 4449 5026 5598 6024 6222 6373 6544 6677 6828 7081 7310	$1798 \left 2030 \right 2260 \left 2490 \right 3058 \left 3620 \right 4179 \right 4842 \left 5391 \right 5936 \left \overline{6477} \right 6895 \left 7115 \right 7284 \right 7446 \left 7597 \right 7902 \left 8154 \right 7446 \left 7597 \right 7902 \left 8154 \right 7667 \left 7667 \right 7667 \left 7$	2160 2422 2680 3325 3834 4467 5094 5715 6333 6946 7555 7869 8071 8224 8440 8739 9038 8071 8224 8440 8739 9038 9038 9038 9038 9038 9038 9038 90
100	per]	inforc	34.					1770	1976	2404	2871	3363	3878	4150	4534	4874	5026	5391	5715
te = 7	onnds	eel Re	04.				1345	1707	1899	2316	2765	3240	3583	3688	4071	4338	4449	4842	5094
ncre	Foot F	s of St	38,			696	1286	1636	1821	2217	2572	2857	3156	3290	3525	3796	3966	4179	4467
8, C	ce in	Inche	.30		828	924	872 961 1150 1223 1286 1345	918 1015 1112 1376 1555 1636 1707 1770	961 1073 1211 1484 1724 1821 1899 1976 2038	2015	2234	2445	2724	2821	3054	3341	3378	3620	3834
puno	esistan	quare	:25	406	628	872	1150	1376	1484	1671	1892	2080	5289	2346	2577	2789	2887	3058	3325
00 P	of R	ea in S	.30	878	619	810	196	1112	1211	1358	1502	1659	1848	1935	2097	2232	2288	2490	2680
20,0	ments	nal Ar	.18	367	260	740		1015	1073	1216	1371	1499	1657	1728	1853	2045	2087	2260	2422
= lee	Me	Section	.16	353	529	650	781			1110	1239	1338	1464	1519	1690	1762	1886	2030	2160
s: St		Cross	.14	338	469	572	069	795	849		1063	1177	1271	1379	1445	1571	1682	1798	
			.12	310	409	494	597	697	736	824	928	1014	1142		1280	1383		-	
m St			.10	263	339	415	505	572	622	715	793		946	955					
rimu			80.	210	277	334	411	471	508	569	611	684							
Max			90.	162	206	253	297	344	377	421									
			8.	108	142	171	200	240											
	Weight	ă	Foot	30	36	42	48	54	09	99	72	78	84	90	96	102	108	114	120
		f Steel to	of Slab	%	**	*	74	%	1	1	1	1	1	17/	11/4	1,4	11/2	11/2	1,12

NOTE—For sectional areas, weights, etc., of Triangle Mesh Wire Reinforcement, see pages 28-29-30. For examples showing use of tables, see page 35-36. For sectional areas, weights, etc., of American Electrically Welded Fabric see pages 32-33-34.

Table No. 19

Recommended MINIMUM DEPTH of Slabs in Inches

Span in Feet. Mixture 1:2:4

19 50	7.0 7.5	8.5 9.0	0.01 2.6	0.11 0.01	11.0 12.0	0.0				
æ	6.5	8.0.8	9.0	9.5 10	10.5 11	11.5 12.	13.0			
21	0.9	7.5	8.5	0.6	10.01	10.5 1	11.5 1	12.0		
16	5.5	7.0	8.0	8.5	9.2	10.01	10.6	11.0	11.5	13.0
15	5.5	6.5	7.5	8.0	8.5	9.6	10.0	10.5	11.0	11.5
14	6.0	6.0	7.0	7.5	8.0	8.5	0.6	8.5	10.0	10.5
18	0.9	6.6	6.5	7.0	7.5	8.0	8.5	0.6	9.6	10.0
25	4.5	5.0	0.9	6.5	7.0	7.6	8.0	8.5	8.5	0.6
Ħ	4.0	4.5	5.5	0.9	6.5	7.0	7.5	7.5	8.0	8.5
01	4.0	4.5	5.0	5.5	0.9	6.5	6.5	7.0	7.5	7.5
٥	3.5	4.0	4.5	5.0	5.5	5.5	6.0	6.5	6.5	0 1
æ	3.5	3.5	4.0	4.5	5.0	2.0	5.5	5.5	0.9	6.0
۲ ها	8.0	3.5	3.5	4.0	4.6	4.6	0.0	0.0	0.0	5.5
0	3.0	8.0	8.5	8.5	4.0	4.0	4.5	4.5	4.5	0.9
, 10	2.5	8.0	8.0	8.5	8.6	4.0	4.0	4.6	4.5	4.5
7	2.5	3.0	3.0	3.5	3.5	4.0	4.0	4.6	4.5	4.5
Live Load in Pounds per Square Foot	20	100	150	200	250	300	350	400	450	200

ment to be 34 inch above bottom. More covering than this may be used by increasing the depth, the extra weight of The depths given are the recommended minimum total thickness of the slab, assuming the center of the reinforcethis concrete being added to the live load

economical. For depths greater than 8 inches, it is more economical to use reinforced concrete beams with thinner slabs It is always allowable and usually advisable to use depths greater than those here specified, thereby decreasing the amount of reinforcement and increasing the amount of concrete. It is also possible to use somewhat less depths, but not between. These depths may also be used for a carefully graded mixture of 1:2½:5 concrete.

Table No. 20

Volume of Concrete Based on a Barrel of 3.8 Cubic Feet

(Reprinted by permission from Taylor & Thompson's "Concrete, Plain and Reinforced")

						Volume				med Concre l of Cemen	
	oportion y Parts			Proportio by Volun		of Mortar in Terms	Pe	rcentages o	f Voids in l or Gravel	Broken Stor	ne
			Cement	Sand	Stone	of Percen- tage of Volume	50%*	45%†	40%‡	30% §	20% §
ement	Sand	Stone	Barrels	Cubic Feet	Cubic Feet	of Stone	Cubic Feet	Cubic Feet	Cubic Feet	Cubic . Feet	Cubi Feet
	1111111112222222222222222222222222666666	1 2 3 4 5 6 7 8 9 10 11 12 11 2 2 1 2 2 1 2 3 3 1 2 2 1 2 3 3 3 1 2 4 1 1 2 5 5 1 2 6 6 1 2 5 5 1 2 6 6 1 2 6 7 7 1 2 6 6 1 2 7 7 1 2 8 5 5 6 7 8 9 9 10 10 12		$\begin{array}{c} 338838838838838838857577666766776677667766776677667766776$	3.8 7.6 11.4 15.2 22.8 226.8 226.8 30.4 33.8 20.3 38.0 41.8 45.5 5.7 7.7 6.9 9.5 11.4 13.3 31.5 22.17 11.1 19.0 20.9 22.8 11.3 3.1 15.2 17.1 19.0 20.9 22.8 11.7 20.9 22.8 24.7 26.6 26.6 26.6 26.6 26.6 26.6 26.6 26	$\begin{array}{c} 94 \\ 51 \\ 36 \\ 29 \\ 22 \\ 20 \\ 19 \\ 187 \\ 16 \\ 99 \\ 76 \\ 187 \\ 16 \\ 99 \\ 76 \\ 61 \\ 19 \\ 99 \\ 76 \\ 61 \\ 44 \\ 40 \\ 76 \\ 68 \\ 44 \\ 40 \\ 76 \\ 68 \\ 46 \\ 49 \\ 44 \\ 40 \\ 76 \\ 64 \\ 49 \\ 44 \\ 40 \\ 46 \\ 46 \\ 46 \\ 46 \\ 46 \\ 46$	5 . 3 . 7 . 4	5 5 7 8 8 10 0 0	5 7 8 2 10 .6	6 2 9 2 12 2 15 2 18 2 21 1 9.7 11 1 12 6 14 0 12 5 13 9 16 7 21 1 16 8 18 2 19 7 11 1 16 8 18 2 19 7 18 1 19 6 21 0 22 1 23 9 25 4 18 1 19 2 23 9 25 4 27 2 28 2 29 2 20 3 20 3	6 7 10 24 10 13 6 17 10 12 10 10 12 10 10 10 10 10 10 10 10 10 10 10 10 10

Note—Variations in the fineness of the sand and the compacting of the concrete may affect the volumes by 10 per cent in either direction.

^{*}Use 50 per cent column for broken stone screened to uniform size. †Use 45 per cent column for average condition sand for broken stone with dust screened out. ‡Use 40 per cent column for gravel or mixed stone and gravel. \$Use these columns for scientifically graded mixtures.

Volume of Plastic Mortar Made from Different Proportions of Cement and Sand Quantities of Materials per Cubic Yard

Reprinted by permission from Taylor & Thompson's "Concrete, Plain and Reinforced"

Relative	1	Volume of	Compac	cted Plas	stic Morta	ar	Mate	erials for	1 Cubic	Yard Co	mnact P	Plastic
Proportions	From 1	Cu. Ft.	Cement	From	1 Barrel (Cement	1.2.00	Mor	tar Base	d on Barr	el of	idatic
Volume*		ed on Por nent Weig		Bas	sed on Ba of	rrel	3.5 Cu	bic Feet	3.8 Cul	bic Feet†	4 Cul	bic Fee
Cement	108 Pounds per Cubic Foot	100 Pounds per Cubic Foot†	95 Pounds per Cubic Foot	3.5 Cubic Feet	3.8 Cubic Feet	4 Cubic Feet	Packed Cement	Loose Sand	Packed Cement	Loose Sand	Packed Cement	Loose Sand
1 0 1 1 1/4 1 1 1/4 1 1 2/4 1 1 2/4 1 1 3/4 1 1 4/4 1 1 5/4 1 1 5/4 1 6/4 1 7/4	1.48 1.84 2.20 2.56 2.92 3.28 3.64 4.01 4.37 4.73 5.09 5.45	cu. ft. 0.86 1.06 1.42 1.78 2.14 2.50 2.86 3.23 3.59 3.95 4.31 4.67 5.03 5.39 5.76 6.12	cu. ft. 0.80 1.02 1.38 1.74 2.11 2.47 2.83 3.19 3.55 3.91 4.28 4.64 5.00 5.36 5.72 6.08	cu. ft. 3.2 3.9 5.2 6.4 7.7 9.0 10.2 11.5 12.8 14.0 15.3 16.6 17.8 19.1 20.3 21.6	cu.ft. 3.2 4.0 5.4 6.7 8.1 9.5 10.9 12.2 13.6 15.0 16.4 17.7 19.1 20.5 21.9 23.2	cu. ft. 3.2 4.1 5.5 7.0 8.4 9.9 11.3 12.8 14.2 15.6 17.1 18.5 20.0 21.4 22.9 24.3	8 31 6 92 5 22 4 20 3 51 3 01 2 64 2 35 2 12 1 77 1 63 1 52 1 41 1 33 1 25	cu. yd. 0.46 0.68 0.81 0.91 0.98 1.03 1.06 1.10 1.12 1.15 1.16 1.18 1.19 1.21	8.31 6.73 5.01 4.00 3.32 2.84 2.20 1.98 1.65 1.52 1.32 1.32	cu. yd. 0.47 0.71 0.84 0.93 1.00 1.05 1.08 1.11 1.14 1.16 1.18 1.19 1.21 1.21	8.31 6.61 4.88 3.87 3.21 2.74 2.39 2.12 1.90 1.72 1.58 1.46 1.18	0.49 0.72 0.86 0.95 1.01 1.06 1.10 1.13 1.15 1.17 1.19 1.20 1.21

Note—Variations in the fineness of the sand and the cement, and in consistency of the mortar, may affect the values by 10 per cent in either direction.

^{*}Cement as packed by manufacturer, sand loose.

[†]Use these columns ordinarily.

Quantities of Materials for One Cubic Yard of Rammed Concrete Based on a Barrel of 3.8 Cubic Feet

(Reprinted by permission from Taylor & Thompson's "Concrete, Plain and Reinforced")

Proportions by Parts				portio			Percentages of Voids in Broken Stor							Stone	Stone or Gravel				=		
D	y Par	ts		Volun	iles	Volume of Mor-	50%*		45%†			40%	t	30% §		20% §					
Cement	Sand	Stone	Packed Cement	Loose	Loose	tar in Terms of Percent- age of Volume	Cement	Sand	Stone	Cement	Sand	Stone	Cement	Sand	Stone	Cement	Sand	Stone	Cement	Sand	Stone
Ö	Sa	St	bbl.	cu. ft.	cu. ft.	of Stone	bbl.	cu. yd.	cu. yd.	bbl.	cu. yd.	cu. yd.	bbl.	cu. yd.	cu. yd.	bbl.	cu. yd.	cu. yd.	bbl.	cu. yd.	cu. yd.
	111111111111111111111111111111111111111	$\begin{array}{c} 1\\ 2\\ 3\\ 4\\ 5\\ 6\\ 7\\ 8\\ 9\\ 10\\ 11\\ 1\\ 1\\ 2\\ 2\\ 1\\ 2\\ 2\\ 1\\ 2\\ 2\\ 2\\ 2\\ 1\\ 2\\ 2\\ 2\\ 2\\ 2\\ 2\\ 2\\ 2\\ 2\\ 2\\ 2\\ 2\\ 2\\$		5 7 7 7 6 6 7 7 6 6 7 7 6 6 7 7 6 6 7 7 6 6 7 7 6 6 7 7 6 6 7 7 6 6 7	7 6 6 9 1 5 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	36 29 25 22 20 19 18 17 16 15 97 75 61 51 93 76 64 55 49 44 40 76 68 61 56 49 40 76 64 40 40 40 40 40 40 40 40 40 40 40 40 40	$\begin{array}{c} 2.34\\ 2.24\\ 2.27\\ 2.09\\ 2.27\\ 2.09\\ 2.27\\ 2.09\\ 2.27\\ 2.09\\$	$ \begin{array}{c} 0.33 \\ 0.48 \\ 0.44 \\ 0.36 \\ 0.36 \\ 0.34 \\ 0.36 \\ 0.36 \\ 0.36 \\ 0.36 \\ 0.36 \\ 0.36 \\ 0.36 \\ 0.36 \\ 0.36 \\ 0.49 \\ 0.40 \\ 0.41 \\ 0.37 \\ 0.61 \\ 0.52 \\ 0.49 \\ 0.52 \\ 0.40 \\ 0.50 \\ 0.48 \\ 0.46 \\ 0.46 \\ 0.46 \\ 0.46 \\ 0.57 \\ 0.58 \\ 0.46 \\ 0.47 \\ 0.48 \\ 0.40 \\ 0.59 \\ 0.47 \\ 0.48 \\ 0.40 \\ 0.59 \\ 0.47 \\ 0.48 \\ 0.40 \\ 0.59 \\ 0.47 \\ 0.48 \\ 0.40 \\ 0.59 \\ 0.47 \\ 0.48 \\ 0.59 \\ 0.59 \\ 0.58 \\ 0.48 \\ 0$	1.03 0.67 0.80 0.99 0.70 0.80 0.99 0.70 0.80 0.98 0.96 0.91 1.10 0.73 0.80 0.91 0.11 0.73 0.80 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.9	$\begin{array}{c} 2 \ 22 \ 22 \ 400 \\ 2 \ 184 \\ 1 \ 71 \ 184 \\ 1 \ 70 \ 184 \\ 1 \ 191 \ 184 \\ 1 \ 191 \ 184 \\ 1 \ 191 \ 184 \\ 1 \ 191 \ 184 \\ 1 \ 191 \ 184 \\ 1 \ 191 \ 184 \\ 1 \ 191 \ 184 \\ 1 \ 191 \ 191 \ 184 \\ 1 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191 \ 191 \ 191 \ 191 \ 191 \ 191 \ 191 \\ 1 \ 191$	0. 43 0. 38 0. 34 0. 31 0. 51 0. 43 0. 39 0. 30 0. 51 0. 42 0. 39 0. 51 0. 45 0. 42 0. 39 0. 47 0. 42 0. 49 0.	0.98 1.14 	2 97 2 31 2 12 2 2 31 2 12 2 31 1 1 76 1 1 51 1 1 42 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0.420 0.420 0.400 0.370 0.330 0.490 0.400 0.370 0.320 0.450	0.63 0.74 0.82 0.92 0.96 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97	$\begin{array}{c} 2.93\\ 2.22\\$	0.399 0.300	0.82 0.94 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	1.77 1.80 1.10 1.25 1.17 1.18 1.25 1.17 1.18 1.25 1.17 1.18 1.25 1.17 1.18 1.25 1.17 1.18 1.25 1.17 1.18 1.25 1.17 1.18 1.25 1.17 1.18 1.25 1.17 1.18 1.25 1.19 1.19 1.19 1.19 1.19 1.19 1.19 1.1	0 . 25 . 3 . 3 . 3 . 3 . 3 . 3 . 3 . 3 . 3 .	0.75 0.63 0.69 0.77 0.63 0.77 0.63 0.77 0.82 0.77 0.82 0.77 0.82 0.75 0.77 0.82 0.77 0.82 0.77 0.82 0.77 0.77 0.82 0.63 0.76 0.76 0.78 0.77 0.77 0.77 0.80 0.82 0.63 0.70 0.74 0.77 0.80 0.70 0.74 0.77 0.80 0.70 0.

Note—Variations in the fineness of the sand and the compacting of the concrete may affect the quantities 10 per cent in either direction.

^{*}Use 50 per cent columns for broken stone screened to uniter size. †Use 45 per cent columns for average conditions and for broken stone with dust screened out. ‡Use 40 per cent columns for gravel or mixed stone and gravel. ‡Use these columns for scientifically graded mixtures.

Table No. 23
Comparative Sizes of Wire Gauge in Decimals of an Inch

No. of Wire Gauge Wire Gauge Steel Ga	Co.'s Wire luge 9900 (6.15 55 55 55 55 55 56	ndard & S.) 8000 1650 6000 0964 6480 2486 8930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	am or Stubs In Sta	nperial andard* I	d English or London 4540 4250 3800 3400 3000 2840 2590 2380 22380 2200 2030 1800 1650 1480 1340 1200
Wire Gauge Steel Ga 00000000 .4 000000 .4 0000 .3 00 .3 0 .3 1 .2 2 .2 3 .2 4 .2 5 .2 6 .1 7 .1 8 .1 9 .1 10 .1 11 .1 12 .1 13 .0 14 .0 15 .0 16 .0 17 .0 18 .0 19 .0 20 .0 21 .0 22 .0 23 .0 24 .0 25 .0 26 .0 27 .0 28 .0 2	Co.'s Wire luge 9900 (6.15 55 55 55 55 55 56	ndard & S.) 8000 1650 6000 0964 6480 2486 8930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	am or Stubs In Sta	nperial andard* I	or London 4540 4250 3800 3400 3000 2840 2590 2380 2200 2030 1800 1650 1480 1340
Ga 00000000 000000 00000 0000 000 000 0	Wire buge (B.	& S.) S 8000 1650 6000 0964 6480 2486 88930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	Stubs Sta 500 454 425 380 340 300 284 2259 238 220 203 180 165 148 134 120	andard* I 500 . 464 . 432 . 400 . 372 . 348 . 324 . 300 . 276 . 252 . 232 . 212 . 192 . 176 . 160 . 144 . 128 . 116 .	4540 4250 3800 3400 3000 2840 2590 22380 2200 2030 1800 1650 1480 1340
00000000 .4 000000 .4 00000 .3 00 .3 0 .3 1 .2 2 .2 3 .2 4 .2 5 .2 6 .1 7 .1 8 .1 9 .1 10 .1 11 .1 12 .1 13 .0 14 .0 15 .0 16 .0 17 .0 18 .0 19 .0 20 .0 21 .0 22 .0 23 .0 24 .0 25 .0 26 .0 27 .0 28 .0 29 .0	900 615 .5: 305 .5: 938 .4: 625 .4: 310 .3: 830 .2: 830 .2: 837 .2: 837 .2: 837 .2: 837 .2: 838 .1: 920 .1: 770 .1: 620 .1: 483 .1 350 .1: 205 .0:	8000 1650 6000 0964 6480 2486 8930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	500 454 425 380 340 300 284 259 238 220 203 180 165 148 134 120	464 432 400 372 348 324 300 276 252 232 212 192 176 160 144 128 116	4250 3800 3400 3000 2840 2590 2380 2200 2030 1800 1650 1480 1340
000000	615	8000 1650 6000 0964 6480 2486 8930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	500 454 425 380 340 300 284 259 238 220 203 180 165 148 134 120	464 432 400 372 348 324 300 276 252 232 212 192 176 160 144 128 116	4250 3800 3400 3000 2840 2590 2380 2200 2030 1800 1650 1480 1340
000000 .4 0000 .3 00 .3 0 .3 1 .2 2 .2 3 .2 4 .2 5 .2 6 .1 7 .1 8 .1 9 .1 10 .1 11 .1 12 .1 13 .0 14 .0 15 .0 16 .0 17 .0 18 .0 19 .0 20 .0 21 .0 22 .0 23 .0 24 .0 25 .0 26 .0 27 .0 28 .0 29 .0	305 .5 938 .4 625 .4 310 .3 6065 .3 830 .2 625 .2 437 .2 253 .2 6070 .1 920 .1 770 .1 620 .1 483 .1 350 .1 205 .0	1650 6000 0964 6480 2486 8930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	500 454 425 380 340 300 284 259 238 220 203 180 165 148 134 120	432 400 372 348 324 300 276 252 232 212 192 176 160 144 128 116	4250 3800 3400 3000 2840 2590 2380 2200 2030 1800 1650 1480 1340
0000	.938 .4 .625 .4 .310 .3 .065 .3 .830 .2 .625 .2 .437 .2 .253 .2 .920 .1 .770 .1 .620 .1 .483 .1 .350 .1 .205 .0	6000 0964 6480 2486 8930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	454 425 380 340 300 284 259 238 220 203 180 165 148 134 120	400 372 348 324 300 276 252 232 212 192 176 160 144 128 116	4250 3800 3400 3000 2840 2590 2380 2200 2030 1800 1650 1480 1340
000	.625 .4 .310 .3 .065 .3 .830 .2 .625 .2 .437 .2 .253 .2 .070 .1 .920 .1 .770 .1 .620 .1 .483 .1 .350 .1 .205 .0	0964 6480 2486 8930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	425 380 340 300 284 259 238 220 203 180 165 148 134 120	372 348 324 300 276 252 232 212 192 176 160 144 128 116	4250 3800 3400 3000 2840 2590 2380 2200 2030 1800 1650 1480 1340
00	310	6480 2486 8930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	380 340 300 284 259 238 220 203 180 165 148 134 120	348 324 300 276 252 232 212 192 176 160 144 128 116	3800 3400 3000 2840 2590 2380 2200 2030 1800 1650 1480 1340
0	.065 .3: .830 .2: .625 .2: .437 .2: .253 .2: .070 .1: .920 .1: .770 .1: .620 .1: .483 .1 .350 .1: .205 .0:	2486 8930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	340 300 284 259 238 220 203 180 165 148 134 120	324 300 276 252 232 212 192 176 160 144 128 116	3400 3000 2840 2590 2380 2200 2030 1800 1650 1480 1340
1	.830 .2 .625 .2 .437 .2 .253 .2 .070 .1 .920 .1 .770 .1 .620 .1 .483 .1 .350 .1	8930 5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	300 284 259 238 220 203 180 165 148 134 120	300 276 252 232 212 192 176 160 144 128 116	3000 2840 2590 2380 2200 2030 1800 1650 1480 1340
2	1625	5763 2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	284 259 238 220 203 180 165 148 134 120	276 252 232 212 192 176 160 144 128 116	2840 2590 2380 2200 2030 1800 1650 1480 1340
3	4437 .2 4253 .2 6070 .1 920 .1 770 .1 620 .1 483 .1 350 .1 205 .0	2942 0431 8194 6202 4428 2849 1443 0189 9074 8081	259 238 220 203 180 165 148 134 120	252 232 212 192 176 160 144 128 116	2590 2380 2200 2030 1800 1650 1480 1340
4 .2 5 .2 6 .1 7 .1 8 .1 9 .1 10 .1 11 .1 12 .1 13 .0 14 .0 15 .0 16 .0 17 .0 18 .0 20 .0 21 .0 22 .0 23 .0 24 .0 25 .0 26 .0 27 .0 28 .0 29 .0	2253 .2070 .11 920 .11 770 .11 620 .11 350 .11 205 .0	0431 8194 6202 4428 2849 1443 0189 9074 8081	238 220 203 180 165 148 134 120	232 212 192 176 160 144 128 116	2380 2200 2030 1800 1650 1480 1340
5	070 .19920 .19770 .19620 .19483 .19350 .19205 .00	8194 6202 4428 2849 1443 0189 9074 8081	220	212 192 176 160 144 128 116	2200 2030 1800 1650 1480 1340
6	920 .10 770 .11 620 .11 483 .1 350 .10 205 .0	6202 4428 2849 1443 0189 9074 8081	203 180 165 148 134 120	192 176 160 144 128 116	2030 1800 1650 1480 1340
7	770	4428 2849 1443 0189 9074 8081	180 165 148 134 120	176 160 144 128 116	1800 1650 1480 1340
8	620 .1: 483 .1: 350 .1: 205 .0:	2849 1443 0189 9074 8081	165 148 134 120	160 144 128 116	1650 1480 1340
9 10 11 11 11 12 11 13 14 10 15 10 16 17 18 19 20 20 21 22 23 24 25 26 27 28 29 .0	483 .1 350 .1 205 .0	1443 0189 9074 8081	148 134 120	144 128 116	1480 1340
10	350 205 .0	0189 9074 8081	134 120	128 116	1340
11	205 .0	9074 . 8081 .	120 .	116 .	
12		8081 .			1.7(1()
13	055		100	104	
14					1090
15					0950
16					0830
17					0720
18					0650
19					0580
20					0490
21 .0 22 .0 23 .0 24 .0 25 .0 26 .0 27 .0 28 .0 29 .0					0400
22	0348 .0	3196 .			0350
23 .0 24 .0 25 .0 26 .0 27 .0 28 .0 29 .0		2846 .			0315
$\begin{array}{c cccc} 24 & & .0 \\ 25 & & .0 \\ 26 & & .0 \\ 27 & & .0 \\ 28 & & .0 \\ 29 & & .0 \end{array}$	0286 .0	2535 .	.028	028 .	0295
25 26 27 28 29)258 .0	2257 .	.025	024 .	0270
26 .0 27 .0 28 .0 29 .0)230 .0	2010 .	.022	022 .	0250
27 .0 28 .0 29 .0)204 .0	1790 .	.020	020 .	0230
28 .0 29 .0	.0 .181	1594 .	.018	018 .	0205
29 .0	0173 .0	1420 .	.016	0164 .	01875
	0162 .0	1264 .	.014	0148 .	01650
30 .0	0150 .0	1126 .	.013	0136 .	01550
	0140 .0	1003 .	.012	0124 .	01375
31 .0	0132 .0	0893 .	.010	0116	01225
					01125
	1110	0708 .			01025
34 .0	0.118	0630 .	.007	0092	00950
					00900
	0104 .0				00750
	0104 .0	0500			00650
	0104 .0 0095 .0 0090 .0	$\begin{array}{c c} 0500 & . \\ 0445 & . \\ \end{array}$			00575
	0104 .0 0095 .0 0090 .0 0085 .0	0445 .			00500
40 .0	0104 .0 0095 .0 0090 .0 0085 .0 0080 .0				00.700

^{*}Also called New British or English Legal Standard.

Table No. 24

American Steel & Wire Co.'s Steel Wire Gauge and
Different Sizes of Wire

	Am. Steel &	D: .	1 6	D 1	D. 1	F 4
Diameter Inches	Wire Co.'s Steel Wire	Diameter Inches	Area Sq. Inches	Pounds Per Foot	Pounds Per Mile	Feet Per Pound
Inches	Gauge	Thenes	Thenes	Ter Poot	1 CI WITH	r cr r ound
-1/			10005	- 0000	2501	1 500
1/2	_	.5000	.19635	.6668	3521.	1.500
15	7	.4900	.18857	.6404	3381.	1.562
$\frac{15}{32}$.46875	.17257	.5861	3094.	1.706
_	8	.4615	.16728	. 5681	2999.	1.76
7 16		.4375	.15033	.5105	2696.	1.959
	5	.4305	.14556	.4943	2610.	2.023
13		.40625	.12962	.4402	2324.	2.272
	40	.3938	.12180	.4136	2184.	2.418
3/8		.3750	.11045	.3751	1980.	2.666
	8 0	.3625	.10321	.3505	1851.	2.853
$\frac{11}{32}$.34375	.092806	.3152	1664.	3.173
	8	.3310	. 086049	. 2922	1543.	3.422
16		.3125	.076699	.2605	1375.	3.839
	0	.3065	.073782	.2506	1323.	3.991
	1	.2830	.062902	.2136	1128.	4.681
37		.28125	.062126	.2110	1114.	4.74
	2	. 2625	.054119	.1838	970.4	5.441
1/4		.2500	.049087	.1667	880.2	5.999
-	3	. 2437	.046645	.1584	836.4	6.313
	4	.2253	.039867	.1354	714.8	7.386
37		.21875	.037583	.1276	673.9	7.835
••	5	.2070	.033654	.1143	603.4	8.750
	6	.1920	.028953	.09832	519.2	10.17
3 16		.1875	.027612	.09377	495.1	10.66
16	7	.1770	.024606	.08356	441.2	11.97
	8	.1620	.020612	.07000	369.6	14.29
5 32		.15625	.019175	.06512	343.8	15.36
••	9	.1483	.017273	.05866	309.7	17.05
	10	.1350	.014314	.04861	256.7	20.57
1/8		.125	.012272•	.04168	220.0	24.00
/8	11	.1205	.011404	.03873	204.5	25.82
	12	.1055	.0087147	.02969	156.7	33.69
17	1-	.09375	.0069029	.02344	123.8	42.66
82	13	.0915	.0065755	.02233	117.9	44.78
	14	.0800	.0050266	.01707	90.13	58.58
	15	.0720	.0040715	.01383	73.01	72.32
	16	.0625	.0030680	.01042	55.01	95.98
	17	.0540	.0022902	.007778	41.07	128.60

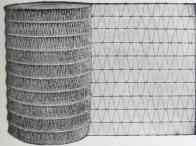
Table No. 25

Weights and Areas of Square and Round Bars and Circumferences of Round Bars

One Cubic Foot of Steel Weighing 489.6 Pounds

Thickness or Diameter	Weight of Square Bar 1 Foot Long	Weight of Round Bar 1 Foot Long	Area of Square Bar in Square	Area of Round Bar in Square	Circumference of Round Bar
in Inches	1 Foot Long	1 Foot Long	Inches	Inches	in Inches
0					
$\frac{1}{16}$.013	.010	.0039	.0031	.1963
1/8	. 053	.042	.0156	.0123	.3927
$ \begin{array}{c} \frac{1}{16} \\ \frac{1}{8} \\ \frac{3}{16} \end{array} $.119	.094	.0352	.0276	. 5890
$\frac{\frac{1}{4}}{\frac{5}{16}}$ $\frac{3}{8}$ $\frac{7}{16}$.212	.167	.0625	.0491	.7854
16	.333	.261	.0977	. 0,767	.9817
3/8	.478	.375	.1406	.1104	1.1781
16	.651	.511	.1914	.1503	1.3744
$\frac{1}{2}$ $\frac{9}{16}$ $\frac{5}{8}$ $\frac{11}{16}$.850	.667	. 2500	.1963	1.5708
16	1.076	. 845	.3164	.2485	1.7671
3/8	1.328	1.043	.3906	.3068	1.9635
16	1.608	1.262	.4727	.3712	2.1598
$\frac{3}{4}$ $\frac{13}{16}$ $\frac{7}{8}$ $\frac{15}{16}$	1.913	1.502	. 5625	.4418	2.3562
13	2.245	1.763	.6602	.5185	2.5525
18	2.603	2.044	.7656	.6013	2.7489
16	2.989	2.347	.8789	. 6903	2.9452
1	3.400	2.670	1.0000	.7854	3.1416
16	3.838	3.014	1.1289	.8866	3.3379
$\frac{1}{16}$ $\frac{1}{8}$ $\frac{3}{16}$	4.303	3.379	1.2656	.9940	3.5343
16	4.795	3.766	1.4102	1.1075	3.7306
$\frac{1/4}{\frac{5}{16}}$ $\frac{3}{8}$ $\frac{7}{16}$	5.312	4.173	1.5625	1.2272	3.9270
16	5.857	4.600	1.7227	1.3530	4.1233
78	6.428	5.049	1.8906	1.4849	4.3197
16	7.026	5.518	2.0664	1.6230	4.5160
$\frac{1/2}{\frac{9}{16}}$ $\frac{5}{8}$ $\frac{11}{16}$	7.650	6.008	2.2500	1.7671	4.7124
16	8.301	6.520	2.4414	1.9175	4.9087
11	8.978	7.051	2.6406	2.0739	5.1051
16	9.682	7.604	2.8477	2.2365	5.3014
$\frac{3}{4}$ $\frac{3}{16}$ $\frac{7}{8}$ $\frac{15}{16}$	10.41	8.178	3.0625	2.4053	5.4978
16	11.17	8.773	3.2852	2.5802	5.6941
/8	11.95	9.388	3.5156	2.7612	5.8905
16	12.76	10.02	3.7539	2.9483	6.0868

Galvanized Triangle Mesh Reinforcement for Stucco



Style 2L, 2-inch Mesh

Exact Size of Main or Longitudinal Wire

Exact Size of Diagonal Cross Wires

The above cuts show the exact sizes of the wires used in the fabric and the general appearance of the mesh itself. The main or longitudinal wires are spaced 4 inches apart and the diagonal cross wires are spaced two inches apart. This fabric can also be furnished with the cross wires spaced 4 inches instead of 2 inches apart although the closer spacing is recommended.

Galvanized Triangle Mesh is primarily a reinforcement for the stucco. By its use destructive cracks are eliminated and increased stability secured. Its superiority is shown by the large wires thoroughly galvanized, low first cost and ease of installation.

Because of the heavy coating of zinc this reinforcement can be successfully used with either cement or magnesite stucco.

Styles, Size and Spacing of Wires-Weights per Square Yard

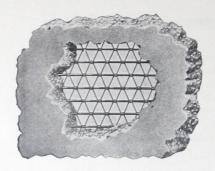
	LONGITUI	DINAL WIRES	CROS	SS WIRES	Approximate Weight Pounds per Sq. Yd.	
Style Number	Gauge	Spacing, Inches	Gauge	Spacing, Inches		
2L	12	4	14	2	2.84	
2M	12	4	14	4	1.94	

Rolls: Made regularly in rolls 150 feet long and 36 or 48 inches wide.

Quantities: Per roll: Rolls 150 feet long, 36 inches wide contain 50 square yards.

Rolls 150 feet long, 48 inches wide contain 66% square yards.





The Essential Importance of Steel Wire Fabric Reinforcement for Concrete Highways

The Reinforcement of Concrete with Steel

Both join into one solid integral mass, the steel imparting its full strength throughout the concrete

Triangle Mesh Reinforcement, shown in above illustration

THE science of modern road-making is based on experience. The need for good roads is now widely realized and appreciated, and wise efforts are being made throughout the country to introduce better road systems. It is important to realize also, what constitutes a good road, and how a road can be built for all time.

From practically every standpoint it has been learned that a permanent, hardsurfaced concrete pavement is the most satisfactory. This is economical as well, because it furnishes a smooth, even, wear-resisting surface for twelve months of the year at a cost that is not excessive.

Properly laid concrete city pavements always include a reinforcement of steel fabric. For the same reason that steel reinforcement is used in the wide city pavements, it should be used in the narrow country roads.

Concrete has the quality of resisting direct wear, but it does not have the power of resisting strain brought about by the uneven settling or heaving of its earth foundation. Substances contract or expand with variation in temperature. Concrete is no exception. Such changes produce a pull that cannot be overcome by the concrete because of its comparatively low tensile strength.

Steel wire fabric having high tensile strength, is therefore placed in the concrete to reduce to a minimum the liability to rupture in the pavement. The continual changing of the concrete slab causes openings unless they are held together by wire fabric.

By the addition of steel wire reinforcing fabric, the originally durable, sightly and sanitary surface of the concrete is kept intact. The additional satisfaction and service obtained from a road reinforced with steel wire fabric makes the extra expense a genuine economy.

Reinforced Concrete Pipe

For Industrial, Sanitary and Storm Water Sewers, Pressure Lines, Highway and Railroad Culverts, Irrigation and Drainage

The main points to be considered in pipe line are strength, economy and efficiency. Proper strength involves thickness of wall, proportion and quality of mixture, amount, type and quality of reinforcement and correct supervision of the manufacture. The thickness of the pipe walls usually is about one-tenth of the pipe diameter. The proportion varies according to local conditions but usually is a 1:2:4 mixture. Triangle Mesh Reinforcement has for years been the standard material for all the principal pipe manufacturers in the United States and Canada including the railroads. The diagonal cross wires have an important bearing on the result as they not only aid in distributing the load, but also prevent the main carrying wires from pulling out through the concrete when the pipe is excessively loaded. No additional reinforcement need be placed lengthwise of the pipe.

Economy involves first cost, installation cost and durability. Precast reinforced concrete pipe is competitive in first cost and shows a decided saving in placing cost especially for unusual ditch conditions.

Many years of successful use have demonstrated the durability of properly made reinforced concrete pipe in the presence of sewer gases, industrial wastes and alkali soils.

Specify reinforced concrete pipe made by experienced manufacturers and reinforced with American Steel & Wire Company's Triangle Mesh Reinforcement.



48 Inch Reinforced Concrete Pipe-Reinforced with Triangle Mesh Reinforcement,

Sizes of Wires

Decimal Equivalents	Full Sizes of	Plain Wire	American Steel & Wire Company's	Sizes (Inc	of Wire	Milli- meters	Weight One Mile	Pounds	Feet
Decimal Equivalents			STEEL WIRE GAUGE No.	Common Fractions	Decimally	(Deci-	(Pounds)	Per Foot	Pound
			1		.2830	7.188	1128.0	.2136	4.681
$\frac{1}{64}$ = .0156 $\frac{33}{64}$ = .515				9 32	. 28125		1114.0	.211	
$\frac{1}{32}$		1900000	2		. 2625	6.668	970.4	.1838	5.441
$\frac{3}{64}$ = .0468 $\frac{35}{64}$ = .546	3			1/4	. 250		880.2	.1667	0.111
$\frac{1}{16}$ = .0625 $\frac{9}{16}$ = .562			3		. 2437	6.190	836.4	.1584	6.313
$\frac{8}{64}$ = .0781 $\frac{37}{64}$ = .578					0050				
3=.0937 3=.593			4	7 32	.2253	5.723	714.8 673.9	.1354	7.386
$\frac{7}{64}$ = .1093 $\frac{39}{64}$ = .6093			5	32	.2070	5.258	603.4	.1143	8.750
½=.125 ½=.625						0.200	000.1	.1110	0.100
$\frac{9}{64}$ 1406 $\frac{41}{64}$ 6406			6		.1920	4.877	519.2	.0983	10.17
$\frac{5}{32}$ = .1562 $\frac{21}{32}$ = .6562			-	3/16	.1875		495.1	.0937	
-			7		.1770	4.496	441.2	.0835	11.97
1718 436718			8	5	.1620	4.115	369.6	.070	14.29
$\frac{3}{16}$ = .1875 $\frac{11}{16}$ = .6875				32	.15625	0 505	343.8	.0651	
$\frac{13}{64}$ = .2031 $\frac{45}{64}$ = .7031			9		.1483	3.767	309.7 256.7	.0586	17.05
$\frac{7}{32}$ = .2187 $\frac{23}{32}$ = .7187			10	1/8	.1250	3.429	220.0	.0486	20.57
$\frac{15}{64}$ = .2343 $\frac{47}{64}$ = .7343			11		.1205	3.061	204.5	.0387	25.82
1/4=.25 3/4=.75	0		12	.	.1055	2.680	156.7	.0296	33.69
$\frac{17}{64}$	0 1		13	17	.09375	2.324	123.8 117.9	.0234	44.78
9 2812 35 7812	0 1		14		.0800	2.032	90.13	.0170	58.58
19 2968 51 7968			15		.0720	1.829	73.01	.0138	72.32
$\frac{5}{16}$ = .3125 $\frac{13}{16}$ = .8125			16	1/6	.0625	1.588	55.0	.0104	95.98
$\frac{21}{64}$ = .3281 $\frac{53}{64}$ = .8281	0 1		17		.0540	1.372	41.07	.0077	128.6
11 = .3437 27 = .8437	0 :		18		.0475	1.207	31.77	.006	166.2
22	• 1		19		.0410	1.041	23.67	.0044	223.0
	• '		20		.0348	.8839	17.05	.0032	309.6
%=.375 7/8=.875	-223		21 22		.0317	.8052	14.15 11.52	.002680 $.002182$	373.1 458.4
$\frac{25}{64}$ = .8906			23		.0258	. 6553	9.374	.001775	563.3
$\frac{13}{32}$ = .4062 $\frac{29}{32}$ = .9062			21 25		0230	.5842	7.450 5.861	.001411	708.7 900.9
₹₹=.4218 55 =.9218			26		.0181	.4597	4.614	.0008738	1144.
7-4375 15-9375			27 28		0173	.4394	4.215 3.696	0007983 0007000	
22 =.4531 61=.9531			29 30		0150	.3810	3.169	.0006001	1666.
15 4687 31 9687			31		0140	.3556	2.760 2.454	0.0005228 0.0004647	
		200	32 33		0128	.3251	2.307 1.961	.0004370	2288.
11=.4843 63=.9843	133	52	34		0104	.2642	1.523	.0003714 .0002885	3466.
1=1.0		- 1	35 36		0095	.2413	1.271	0002407 0002160	4154.
			00 1	1.	0000	.22001	1.1411	.0002160	4029.



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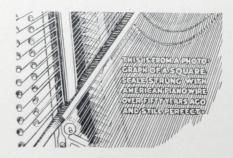
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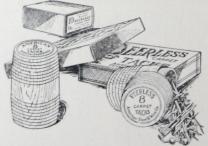
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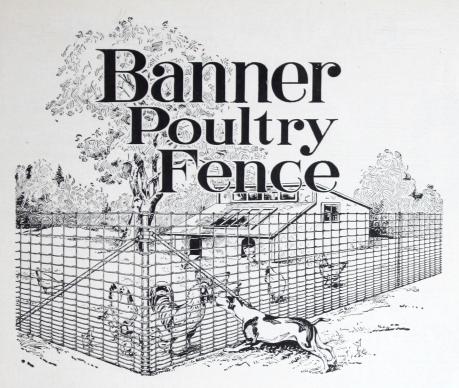
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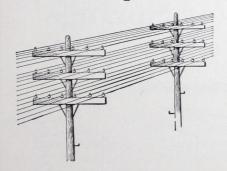
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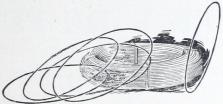
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